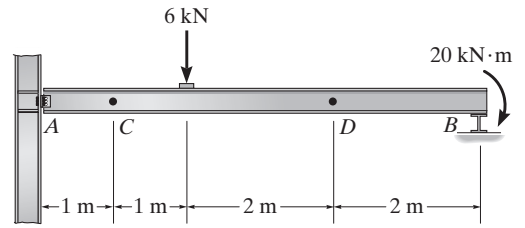


4-1. Determine the internal normal force, shear force, and bending moment in the beam at points *C* and *D*. Assume the support at *A* is a pin and *B* is a roller.



Entire beam:

$$\rightarrow \sum F_x = 0; \quad A_x = 0$$

$$\zeta + \sum M_A = 0; \quad B_y(6) - 20 - 6(2) = 0$$

$$B_y = 5.333 \text{ kN}$$

$$+\uparrow \sum F_y = 0; \quad A_y + 5.333 - 6 = 0$$

$$A_y = 0.6667 \text{ kN}$$

Segment *AC*:

$$\rightarrow \sum F_x = 0; \quad N_C = 0$$

$$+\uparrow \sum F_y = 0; \quad 0.6667 - V_C = 0$$

$$V_C = 0.667 \text{ kN}$$

$$\zeta + \sum M_C = 0; \quad M_C - 0.6667(1) = 0$$

$$M_C = 0.667 \text{ kN} \cdot \text{m}$$

Segment *DB*:

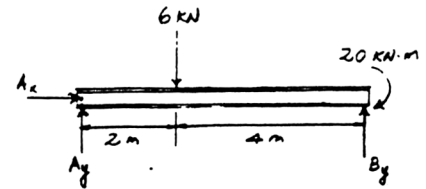
$$\rightarrow \sum F_x = 0; \quad N_D = 0$$

$$+\uparrow \sum F_y = 0; \quad V_D + 5.333 = 0$$

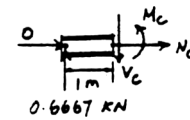
$$V_D = -5.33 \text{ kN}$$

$$\zeta + \sum M_D = 0; \quad -M_D + 5.333(2) - 20 = 0$$

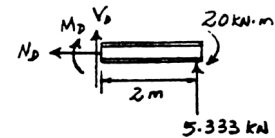
$$M_D = -9.33 \text{ kN} \cdot \text{m}$$



Ans.



Ans.



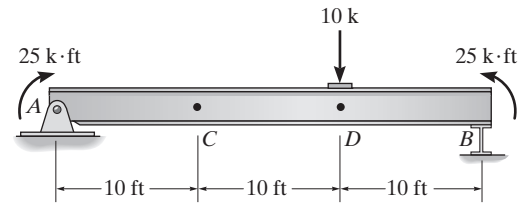
Ans.

Ans.

Ans.

Ans.

4-2. Determine the internal normal force, shear force, and bending moment in the beam at points *C* and *D*. Assume the support at *B* is a roller. Point *D* is located just to the right of the 10-k load.



Entire Beam:

$$\zeta + \sum M_A = 0; \quad B_y(30) + 25 - 25 - 10(20) = 0$$

$$B_y = 6.667 \text{ k}$$

$$+\uparrow \sum F_y = 0; \quad A_y + 6.667 - 10 = 0$$

$$A_y = 3.333 \text{ k}$$

$$\rightarrow \sum F_x = 0; \quad A_x = 0$$

Segment AC:

$$\rightarrow \sum F_x = 0; \quad N_C = 0$$

$$+\uparrow \sum F_y = 0; \quad -V_C + 3.333 = 0$$

$$V_C = 3.33 \text{ k}$$

$$\zeta + \sum M_C = 0; \quad M_C - 25 - 3.333(10) = 0$$

$$M_C = 58.3 \text{ k} \cdot \text{ft}$$

Segment DB:

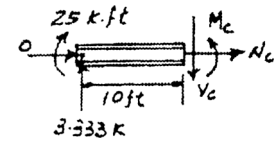
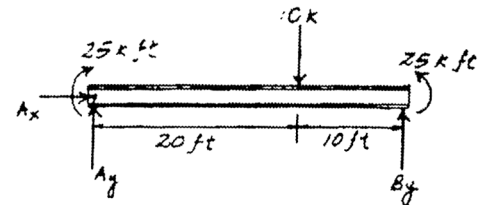
$$\rightarrow \sum F_x = 0; \quad N_D = 0$$

$$+\uparrow \sum F_y = 0; \quad V_D + 6.667 = 0$$

$$V_D = -6.67 \text{ k}$$

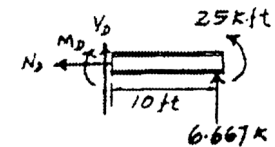
$$\zeta + \sum M_D = 0; \quad -M_D + 25 + 6.667(10) = 0$$

$$M_D = 91.7 \text{ k} \cdot \text{ft}$$



Ans.

Ans.



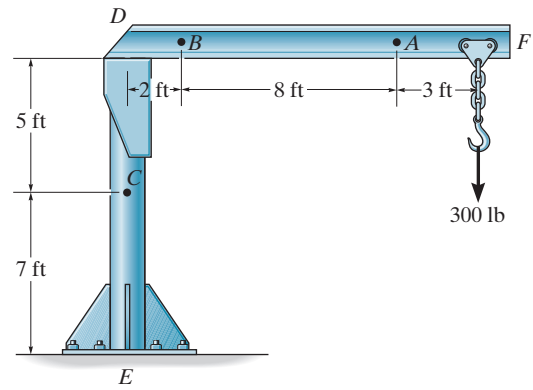
Ans.

Ans.

Ans.

Ans.

4-3. The boom DF of the jib crane and the column DE have a uniform weight of 50 lb/ft. If the hoist and load weigh 300 lb, determine the internal normal force, shear force, and bending moment in the crane at points A , B , and C .



Equations of Equilibrium: For point A

$$\leftarrow \sum F_x = 0;$$

$$N_A = 0$$

$$+\uparrow \sum F_y = 0;$$

$$V_A - 150 - 300 = 0$$

$$V_A = 450 \text{ lb}$$

$$\zeta + \sum M_A = 0;$$

$$-M_A - 150(1.5) - 300(3) = 0$$

$$M_A = -1125 \text{ lb} \cdot \text{ft} = -1.125 \text{ kip} \cdot \text{ft}$$

Negative sign indicates that M_A acts in the opposite direction to that shown on FBD.

Equations of Equilibrium: For point B

$$\leftarrow \sum F_x = 0;$$

$$N_B = 0$$

$$+\uparrow \sum F_y = 0;$$

$$V_B - 550 - 300 = 0$$

$$V_B = 850 \text{ lb}$$

$$\zeta + \sum M_B = 0;$$

$$-M_B - 550(5.5) - 300(11) = 0$$

$$M_B = -6325 \text{ lb} \cdot \text{ft} = -6.325 \text{ kip} \cdot \text{ft}$$

Negative sign indicates that M_B acts in the opposite direction to that shown on FBD.

Equations of Equilibrium: For point C

$$\leftarrow \sum F_x = 0;$$

$$V_C = 0$$

$$+\uparrow \sum F_y = 0;$$

$$-N_C - 250 - 650 - 300 = 0$$

$$N_C = -1200 \text{ lb} = -1.20 \text{ kip}$$

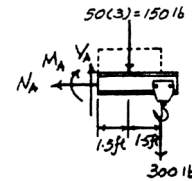
$$\zeta + \sum M_C = 0;$$

$$-M_C - 650(6.5) - 300(13) = 0$$

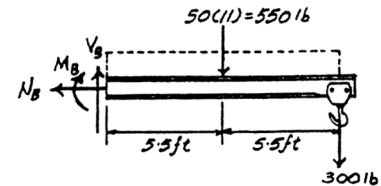
$$M_C = -8125 \text{ lb} \cdot \text{ft} = -8.125 \text{ kip} \cdot \text{ft}$$

Negative sign indicate that N_C and M_C act in the opposite direction to that shown on FBD.

Ans.

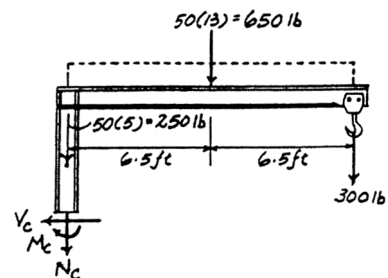


Ans.



Ans.

Ans.

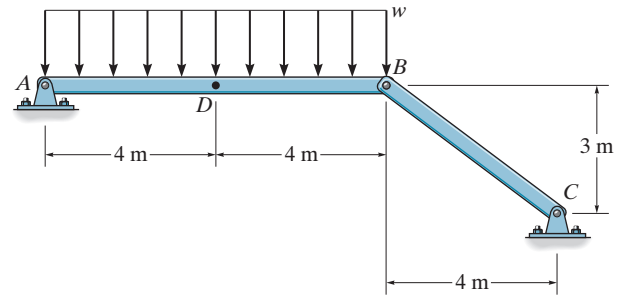


Ans.

Ans.

Ans.

*4-4. Determine the internal normal force, shear force, and bending moment at point D . Take $w = 150 \text{ N/m}$.



$$\zeta + \sum M_A = 0; \quad -150(8)(4) + \frac{3}{5} F_{BC}(8) = 0$$

$$F_{BC} = 1000 \text{ N}$$

$$\rightarrow \sum F_x = 0; \quad A_x - \frac{4}{5}(1000) = 0$$

$$A_x = 800 \text{ N}$$

$$+\uparrow \sum F_y = 0; \quad A_y - 150(8) + \frac{3}{5}(1000) = 0$$

$$A_y = 600 \text{ N}$$

$$\rightarrow \sum F_x = 0; \quad N_D = -800 \text{ N}$$

Ans.

$$+\uparrow \sum F_y = 0; \quad 600 - 150(4) - V_D = 0$$

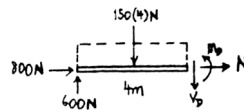
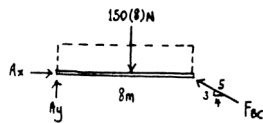
$$V_D = 0$$

Ans.

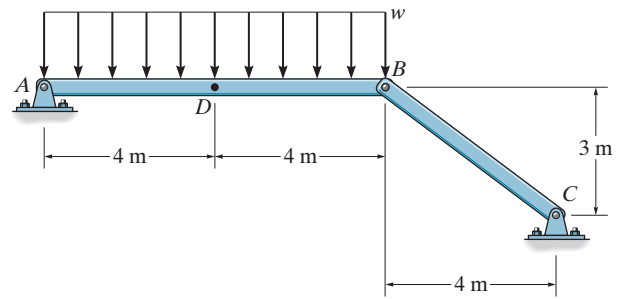
$$\zeta + \sum M_D = 0; \quad -600(4) + 150(4)(2) + M_D = 0$$

$$M_D = 1200 \text{ N} \cdot \text{m} = 1.20 \text{ kN} \cdot \text{m}$$

Ans.



4-5. The beam AB will fail if the maximum internal moment at D reaches $800 \text{ N}\cdot\text{m}$ or the normal force in member BC becomes 1500 N . Determine the largest load w it can support.



Assume maximum moment occurs at D ;

$$\zeta + \sum M_D = 0; \quad M_D - \frac{8w}{2}(4) + 4w(2) = 0$$

$$800 = 8w$$

$$w = 100 \text{ N/m}$$

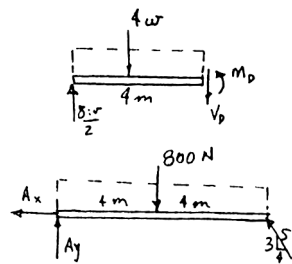
$$\zeta + \sum M_A = 0; \quad -800(4) + T_{BC}(0.6)(8) = 0$$

$$T_{BC} = 666.7 \text{ N} < 1500 \text{ N}$$

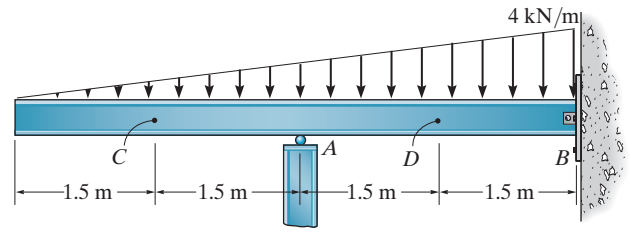
$$w = 100 \text{ N/m}$$

(O. K!)

Ans.



4-6. Determine the internal normal force, shear force, and bending moment in the beam at points C and D. Assume the support at A is a roller and B is a pin.



Support Reactions. Referring to the FBD of the entire beam in Fig. a,

$$\zeta + \sum M_B = 0; \quad \frac{1}{2}(4)(6)(2) - A_y(3) = 0 \quad A_y = 8 \text{ kN}$$

Internal Loadings. Referring to the FBD of the left segment of the beam sectioned through point C, Fig. b,

$$\rightarrow \sum F_x = 0; \quad N_C = 0 \quad \text{Ans.}$$

$$+\uparrow \sum F_y = 0; \quad -\frac{1}{2}(1)(1.5) - V_C = 0 \quad V_C = -0.75 \text{ kN} \quad \text{Ans.}$$

$$\zeta + \sum M_C = 0; \quad M_C + \frac{1}{2}(1)(1.5)(0.5) = 0 \quad M_C = -0.375 \text{ kN}\cdot\text{m} \quad \text{Ans.}$$

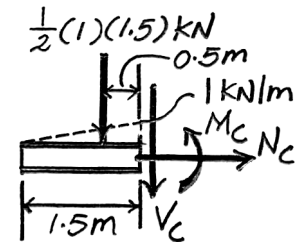
Referring to the FBD of the left segment of the beam sectioned through point D, Fig. c,

$$\rightarrow \sum F_x = 0; \quad N_D = 0 \quad \text{Ans.}$$

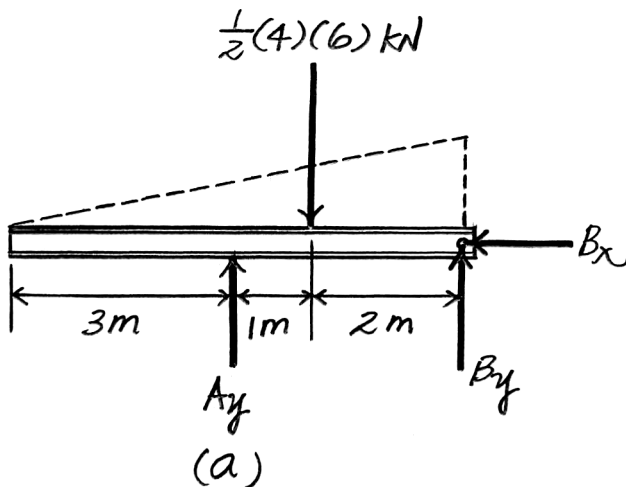
$$+\uparrow \sum F_y = 0; \quad 8 - \frac{1}{2}(3)(4.5) - V_D = 0 \quad V_D = 1.25 \text{ kN} \quad \text{Ans.}$$

$$\zeta + \sum M_D = 0; \quad M_D + \frac{1}{2}(3)(-4.5)(1.5) - 8(1.5) = 0$$

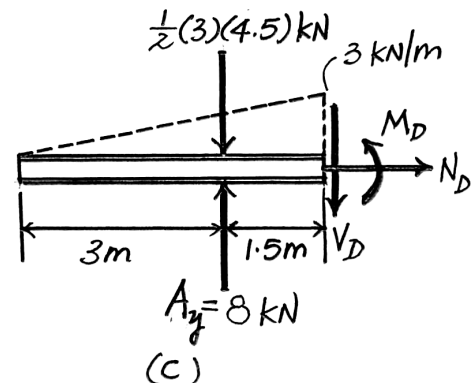
$$M_D = 1.875 \text{ kN}\cdot\text{m} \quad \text{Ans.}$$



(b)

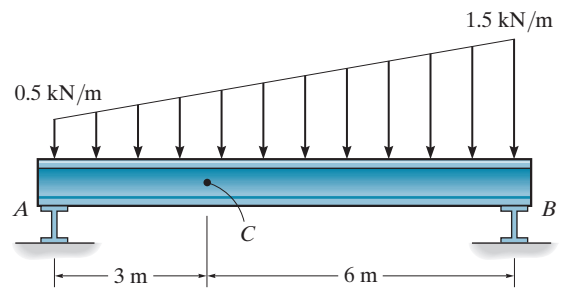


(a)

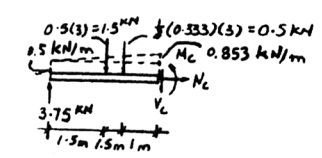
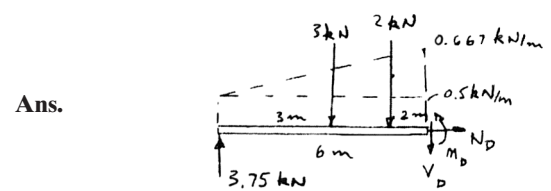
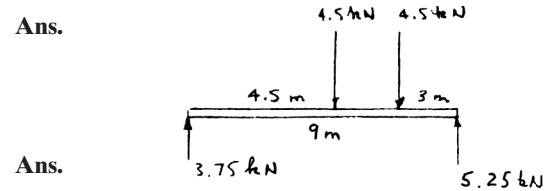


(c)

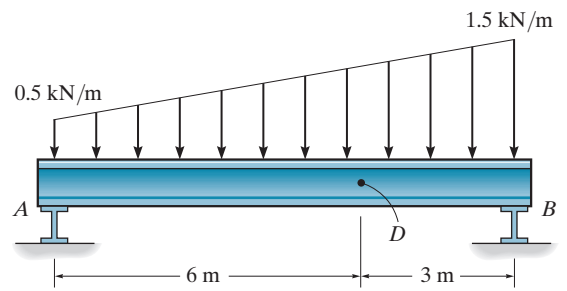
4-7. Determine the internal normal force, shear force, and bending moment at point *C*. Assume the reactions at the supports *A* and *B* are vertical.



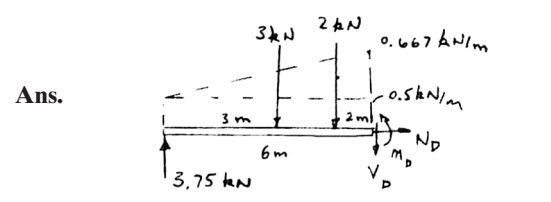
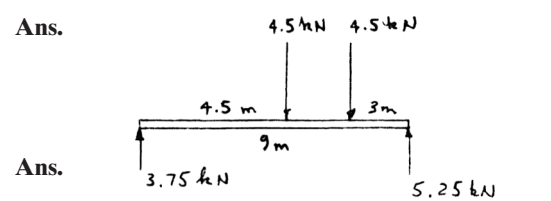
$$\begin{aligned} \rightarrow \sum F_x = 0; & \quad N_C = 0 \\ +\downarrow \sum F_y = 0; & \quad V_C + 0.5 + 1.5 - 3.75 = 0 \\ & \quad V_C = 1.75 \text{ kN} \\ \zeta + \sum M_C = 0; & \quad M_C + 0.5(1) + 1.5(1.5) - 3.75(3) = 0 \\ & \quad M_C = 8.50 \text{ kN}\cdot\text{m} \end{aligned}$$



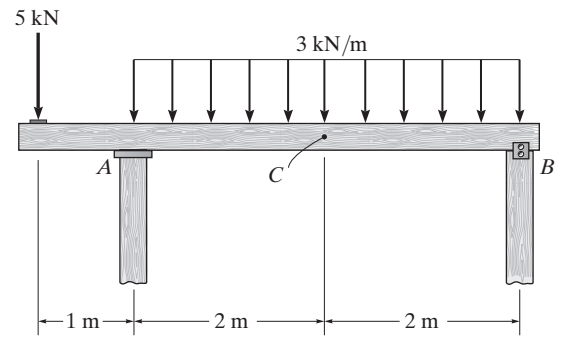
***4-8.** Determine the internal normal force, shear force, and bending moment at point *D*. Assume the reactions at the supports *A* and *B* are vertical.



$$\begin{aligned} \rightarrow \sum F_x = 0; & \quad N_D = 0 \\ +\uparrow \sum F_y = 0; & \quad 3.75 - 3 - 2 - V_D = 0 \\ & \quad V_D = -1.25 \text{ kN} \\ \zeta + \sum M_D = 0; & \quad M_D + 2(2) + 3(3) - 3.75(6) = 0 \\ & \quad M_D = 9.50 \text{ kN}\cdot\text{m} \end{aligned}$$



4-9. Determine the internal normal force, shear force, and bending moment in the beam at point C. The support at A is a roller and B is pinned.



Support Reactions. Referring to the FBD of the entire beam in Fig a,

$$\zeta + \sum M_A = 0; \quad B_y(4) + 5(1) - 3(4)(2) = 0 \quad B_y = 4.75 \text{ kN}$$

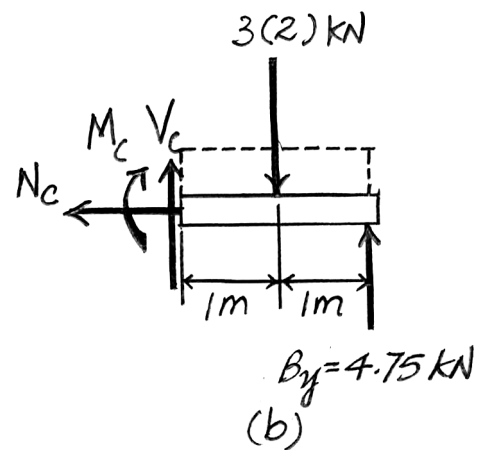
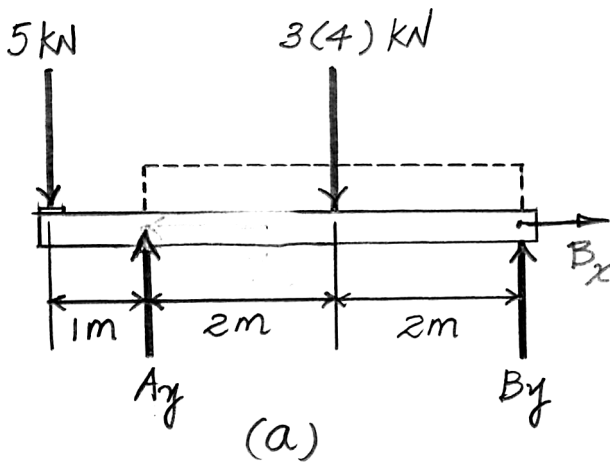
$$\rightarrow \sum F_x = 0; \quad B_x = 0$$

Internal Loadings. Referring to the FBD of the right segment of the beam sectioned through point c, Fig. b,

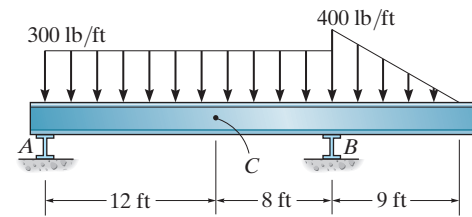
$$\rightarrow \sum F_x = 0; \quad N_c = 0 \quad \text{Ans.}$$

$$+\uparrow \sum F_y = 0; \quad V_c + 4.75 - 3(2) = 0 \quad V_c = 1.25 \text{ kN} \quad \text{Ans.}$$

$$\zeta + \sum M_c = 0; \quad 4.75(2) - 3(2)(1) - M_c = 0 \quad M_c = 3.50 \text{ kN} \cdot \text{m} \quad \text{Ans.}$$



4-10. Determine the internal normal force, shear force, and bending moment at point *C*. Assume the reactions at the supports *A* and *B* are vertical.



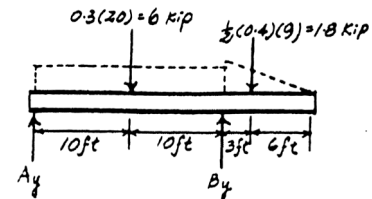
Support Reactions:

$$\zeta + \sum M_A = 0; \quad B_y(20) - 6(10) - 1.8(23) = 0$$

$$B_y = 5.07 \text{ kip}$$

$$+\uparrow \sum F_y = 0; \quad A_y + 5.07 - 6 - 1.8 = 0$$

$$A_y = 2.73 \text{ kip}$$



Equations of Equilibrium: For point *C*

$$\rightarrow \sum F_x = 0; \quad N_C = 0$$

Ans.

$$+\uparrow \sum F_y = 0; \quad 2.73 - 3.60 - V_C = 0$$

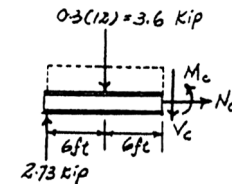
$$V_C = -0.870 \text{ kip}$$

Ans.

$$\zeta + \sum M_C = 0; \quad M_C + 3.60(6) - 2.73(12) = 0$$

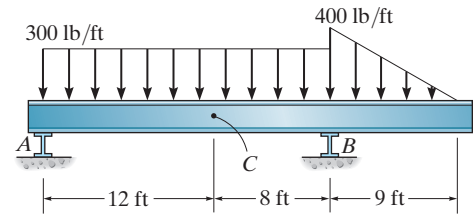
$$M_C = 11.2 \text{ kip} \cdot \text{ft}$$

Ans.



Negative sign indicates that V_C acts in the opposite direction to that shown on FBD.

4-11. Determine the internal normal force, shear force, and bending moment at points *D* and *E*. Assume the reactions at the supports *A* and *B* are vertical.



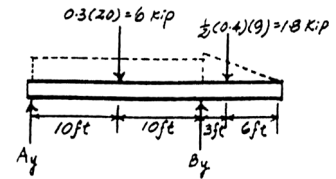
Support Reactions:

$$\zeta + \sum M_A = 0; \quad B_y(20) - 6(10) - 1.8(23) = 0$$

$$B_y = 5.07 \text{ kip}$$

$$+\uparrow \sum F_y = 0; \quad A_y + 5.07 - 6 - 1.8 = 0$$

$$A_y = 2.73 \text{ kip}$$



Equations of Equilibrium: For point *D*

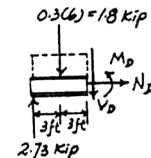
$$\rightarrow \sum F_x = 0; \quad N_D = 0 \quad \text{Ans.}$$

$$+\uparrow \sum F_y = 0; \quad 2.73 - 1.8 - V_D = 0$$

$$V_D = 0.930 \text{ kip} \quad \text{Ans.}$$

$$\zeta + \sum M_D = 0; \quad M_D + 1.8(3) - 2.73(6) = 0$$

$$M_D = 11.0 \text{ kip} \cdot \text{ft} \quad \text{Ans.}$$



Equations of Equilibrium: For point *E*

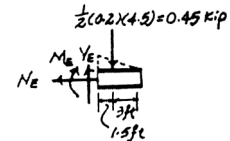
$$\leftarrow \sum F_x = 0; \quad N_E = 0 \quad \text{Ans.}$$

$$+\uparrow \sum F_y = 0; \quad V_E - 0.45 = 0$$

$$V_E = 0.450 \text{ kip} \quad \text{Ans.}$$

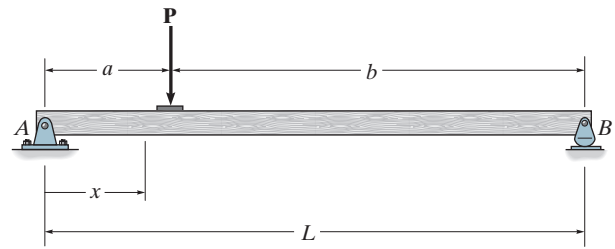
$$\zeta + \sum M_E = 0; \quad -M_E - 0.45(1.5) = 0$$

$$M_E = -0.675 \text{ kip} \cdot \text{ft} \quad \text{Ans.}$$



Negative sign indicates that M_E acts in the opposite direction to that shown on FBD.

*4-12. Determine the shear and moment throughout the beam as a function of x .



Support Reactions: Referring to the FBD of the entire beam in Fig. a,

$$\zeta + \sum M_A = 0; \quad N_B(L) - Pa = 0 \quad N_B = \frac{Pa}{L}$$

$$\zeta + \sum M_B = 0; \quad Pb - A_y(L) = 0 \quad A_y = \frac{Pb}{L}$$

$$\rightarrow \sum F_x = 0; \quad A_x = 0$$

Internal Loading: For $0 \leq x < a$, refer to the FBD of the left segment of the beam in Fig. b.

$$+\uparrow \sum F_y = 0; \quad \frac{Pb}{L} - V = 0 \quad V = \frac{Pb}{L}$$

Ans.

$$\zeta + \sum M_O = 0; \quad M - \frac{Pb}{L}x = 0 \quad M = \frac{Pb}{L}x$$

Ans.

For $a < x \leq L$, refer to the FBD of the right segment of the beam in Fig. c.

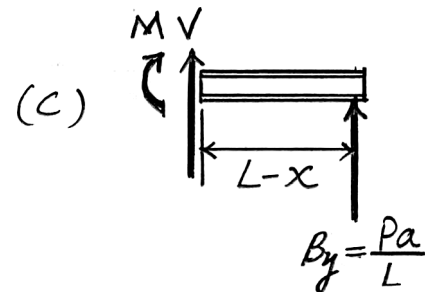
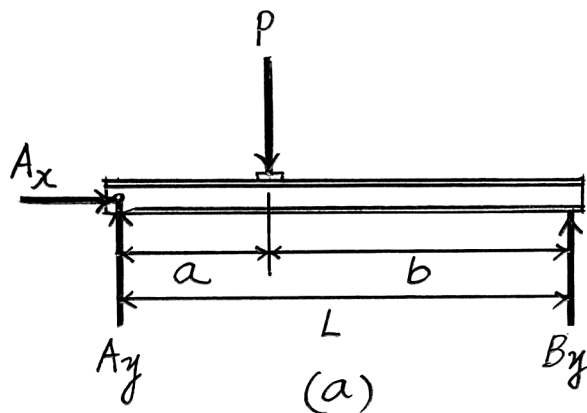
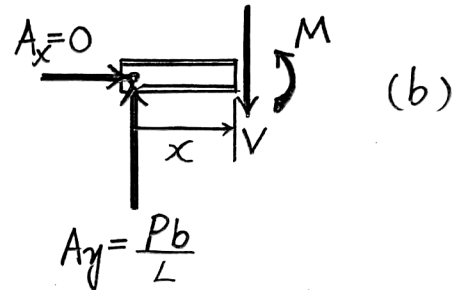
$$+\uparrow \sum F_y = 0; \quad V + \frac{Pa}{L} = 0 \quad V = -\frac{Pa}{L}$$

Ans.

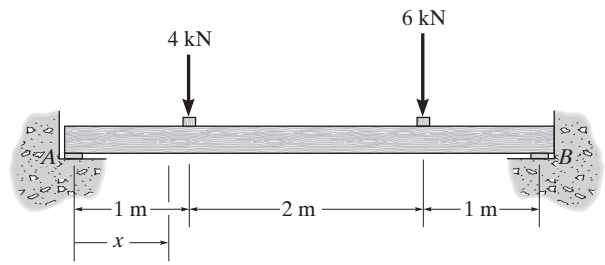
$$\zeta + \sum M_O = 0; \quad \frac{Pa}{L}(L-x) - M = 0$$

$$M = \frac{Pa}{L}(L-x)$$

Ans.



4-13. Determine the shear and moment in the floor girder as a function of x . Assume the support at A is a pin and B is a roller.



Support Reactions: Referring to the FBD of the entire beam in Fig. *a*.

$$\zeta + \sum M_A = 0; \quad B_y(4) - 4(1) - 6(3) = 0$$

$$B_y = 5.50 \text{ kN}$$

$$\zeta + \sum M_B = 0; \quad 6(1) + 4(3) - A_y(4) = 0$$

$$A_y = 4.50 \text{ kN}$$

$$\rightarrow \sum F_x = 0; \quad A_x = 0$$

Internal Loadings: For $0 \leq x < 1$ m, Referring to the FBD of the left segment of the beam in Fig. *b*,

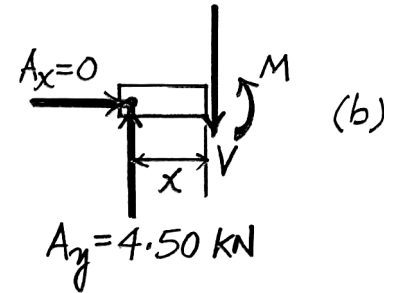
$$+\uparrow \sum F_y = 0; \quad 4.50 - V = 0 \quad V = 4.50 \text{ kN}$$

Ans.

$$\zeta + \sum M_O = 0; \quad M - 4.50x = 0$$

$$M = \{4.50x\} \text{ kN} \cdot \text{m}$$

Ans.



For $1 \text{ m} < x < 3$ m, referring to the FBD of the left segment of the beam in Fig. *c*,

$$+\uparrow \sum F_y = 0; \quad 4.50 - 4 - V = 0$$

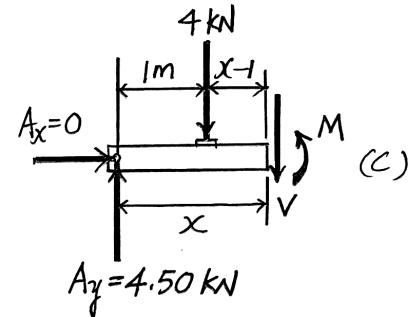
$$V = 0.500 \text{ kN}$$

Ans.

$$\zeta + \sum M_O = 0; \quad M + 4(x-1) - 4.50x = 0$$

$$M = \{0.5x + 4\} \text{ kN} \cdot \text{m}$$

Ans.



For $3 \text{ m} < x \leq 4$ m, referring to the FBD of the right segment of the beam in Fig. *d*,

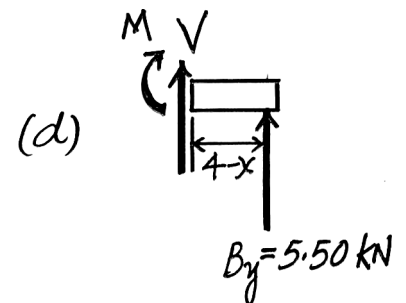
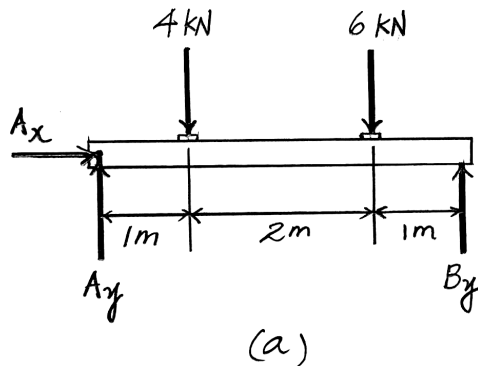
$$+\uparrow \sum F_y = 0; \quad V + 5.50 = 0 \quad V = -5.50 \text{ kN}$$

Ans.

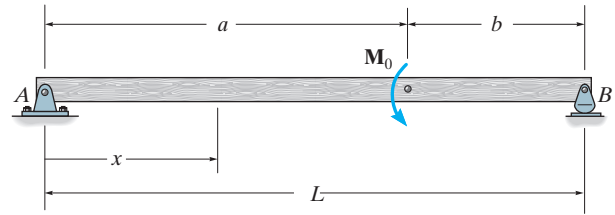
$$\zeta + \sum M_O = 0; \quad 5.50(4-x) - M = 0$$

$$M = \{-5.50x + 22\} \text{ kN} \cdot \text{m}$$

Ans.



4-14. Determine the shear and moment throughout the beam as a function of x .



Support Reactions: Referring to the FBD of the entire beam in Fig. *a*

$$\rightarrow \sum F_x = 0; \quad A_x = 0$$

$$\zeta + \sum M_A = 0; \quad M_0 - N_B(L) = 0 \quad B_y = \frac{M_0}{L}$$

$$\zeta + \sum M_B = 0; \quad M_0 - A_y(L) = 0 \quad A_y = \frac{M_0}{L}$$

Internal Loadings: For $0 \leq x < a$, refer to the FBD of the left segment of the beam is Fig. *b*.

$$+\uparrow \sum F_y = 0; \quad \frac{M_0}{L} - V = 0 \quad V = \frac{M_0}{L} \quad \text{Ans.}$$

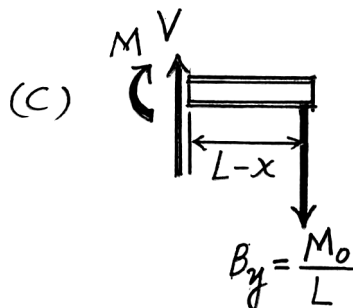
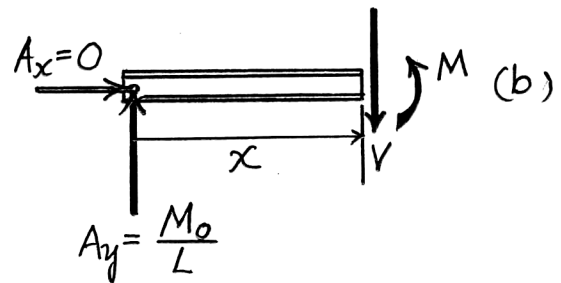
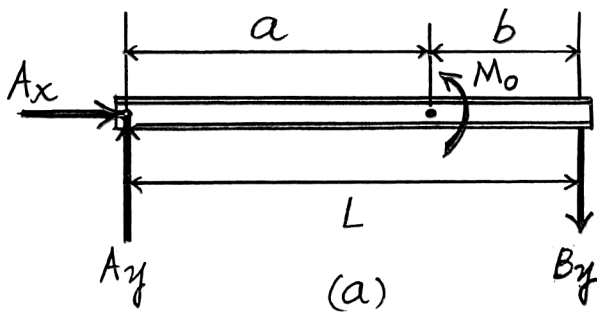
$$\zeta + \sum M_o = 0; \quad M - \frac{M_0}{L}x = 0 \quad M = \frac{M_0}{L}x \quad \text{Ans.}$$

For $a < x \leq L$, refer to the FBD of the right segment of the beam in Fig. *c*

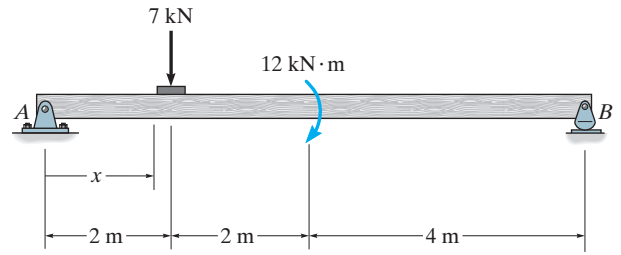
$$+\uparrow \sum F_y = 0; \quad V - \frac{M_0}{L} = 0 \quad V = \frac{M_0}{L} \quad \text{Ans.}$$

$$\zeta + \sum M_o = 0; \quad -M - \frac{M_0}{L}(L - x) = 0 \quad \text{Ans.}$$

$$M = -\frac{M_0}{L}(L - x) \quad \text{Ans.}$$



4-15. Determine the shear and moment throughout the beam as a function of x .



Reaction at A:

$$\rightarrow \sum F_x = 0; \quad A_x = 0$$

$$\zeta + \sum M_B = 0; \quad A_y(8) - 7(6) + 12 = 0; \quad A_y = 3.75 \text{ kN}$$

$0 \leq x < 2 \text{ m}$

$$+\uparrow \sum F_y = 0; \quad 3.75 - V = 0; \quad V = 3.75 \text{ kN}$$

$$\zeta + \sum M = 0; \quad 3.75x - M = 0; \quad M = 3.75x \text{ kN}$$

$2 \text{ m} < x < 4 \text{ m}$

Segment:

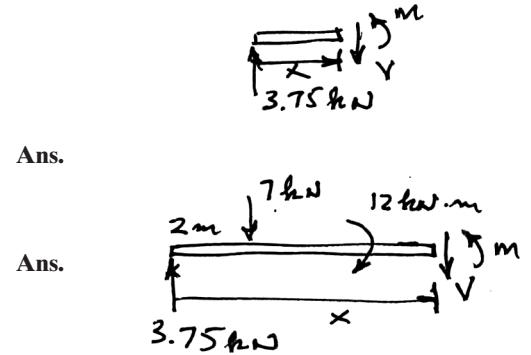
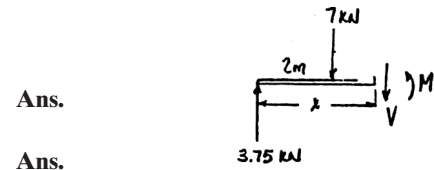
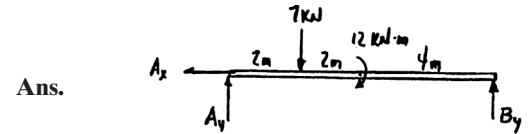
$$+\uparrow \sum F_y = 0; \quad -V + 3.75 - 7 = 0; \quad V = -3.25$$

$$\zeta + \sum M = 0; \quad -M + 3.75x - 7(x - 2) = 0; \quad M = -3.25x + 14$$

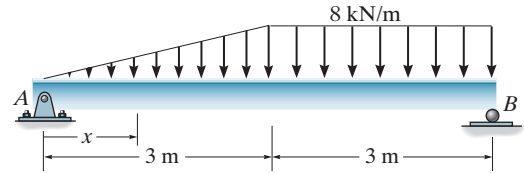
$4 \text{ m} < x \leq 8 \text{ m}$

$$+\uparrow \sum F_y = 0; \quad 3.75 - 7 - V = 0; \quad V = -3.25 \text{ kN}$$

$$\zeta + \sum M = 0; \quad -3.75x + 7(x - 2) - 12 + M = 0; \quad M = 26 - 3.25x$$



*4-16. Determine the shear and moment throughout the beam as a function of x .



Support Reactions. Referring to the FBD of the entire beam in Fig. *a*,

$$\zeta + \sum M_A = 0; \quad B_y(6) - 8(3)(4.5) - \frac{1}{2}(8)(3)(2) = 0$$

$$B_y = 22 \text{ kN}$$

$$\zeta + \sum M_B = 0; \quad 8(3)(1.5) + \frac{1}{2}(8)(3)(4) - A_y(6) = 0$$

$$A_y = 14 \text{ kN}$$

$$\rightarrow \sum F_x = 0; \quad A_x = 0$$

Internal Loadings: For $0 \leq x < 3$ m, refer to the FBD of the left segment of the beam in Fig. *b*,

$$+\uparrow \sum F_y = 0; \quad 14 - \frac{1}{2}\left(\frac{8}{3}x\right)x - V = 0$$

$$V = \{-1.33x^2 + 14\} \text{ kN}$$

Ans.

$$\zeta + \sum M_O = 0; \quad M + \frac{1}{2}\left(\frac{8}{3}x\right)(x)\left(\frac{x}{3}\right) - 14x = 0$$

$$M = \{-0.444x^3 + 14x\} \text{ kN} \cdot \text{m}$$

Ans.

For $3 \text{ m} < x \leq 6$ m, refer to the FBD of the right segment of the beam in Fig. *c*

$$+\uparrow \sum F_y = 0; \quad V + 22 - 8(6-x) = 0$$

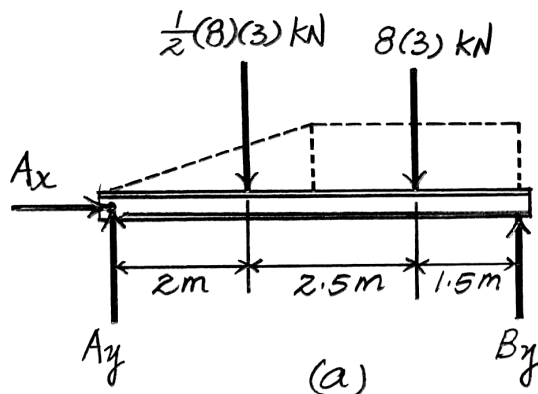
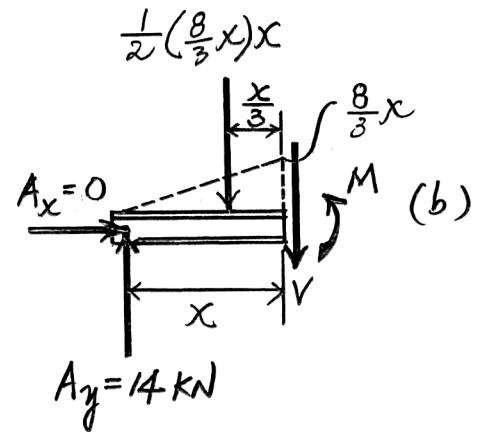
$$V = \{-8x + 26\} \text{ kN}$$

$$\zeta + \sum M_O = 0; \quad 22(6-x) - 8(6-x)\left(\frac{6-x}{2}\right) - M = 0$$

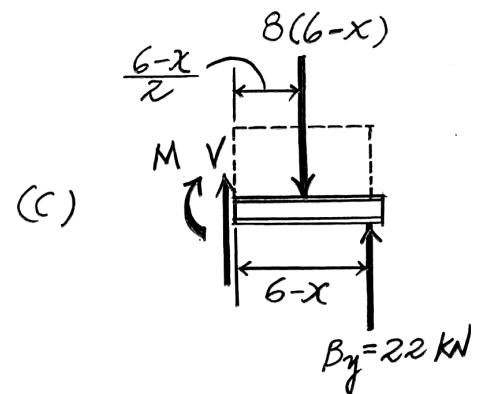
$$M = \{-4x^2 + 26x - 12\} \text{ kN} \cdot \text{m}$$

Ans.

Ans.

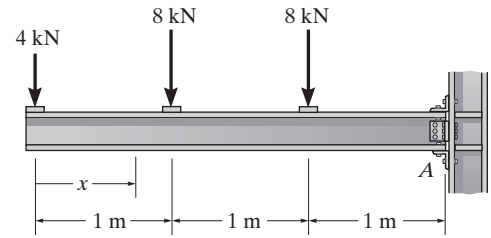


(a)



(c)

4-17. Determine the shear and moment throughout the beam as a function of x .



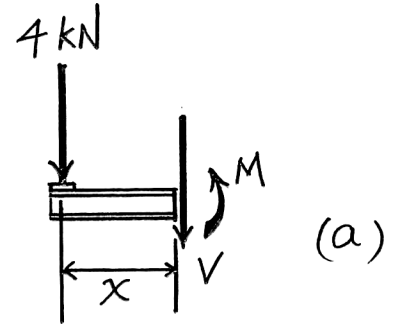
Internal Loadings. For $0 \leq x \leq 1$ m, referring to the FBD of the left segment of the beam in Fig. *a*,

$$+\uparrow \sum F_y = 0; \quad -V - 4 = 0 \quad V = -4 \text{ kN}$$

Ans.

$$\zeta + \sum M_O = 0; \quad M + 4x = 0 \quad M = \{-4x\} \text{ kN}\cdot\text{m}$$

Ans.



For $1 \text{ m} < x < 2 \text{ m}$, referring to the FBD of the left segment of the beam in Fig. *b*,

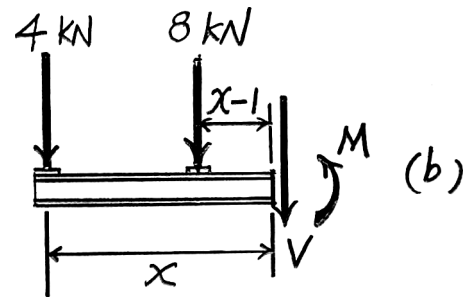
$$+\uparrow \sum F_y = 0; \quad -4 - 8 - V = 0 \quad V = \{-12\} \text{ kN}\cdot\text{m}$$

Ans.

$$\zeta + \sum M_O = 0; \quad M + 8(x - 1) + 4x = 0$$

$$M = \{-12x + 8\} \text{ kN}\cdot\text{m}$$

Ans.



For $2 \text{ m} < x \leq 3 \text{ m}$, referring to the FBD of the left segment of the beam in Fig. *c*,

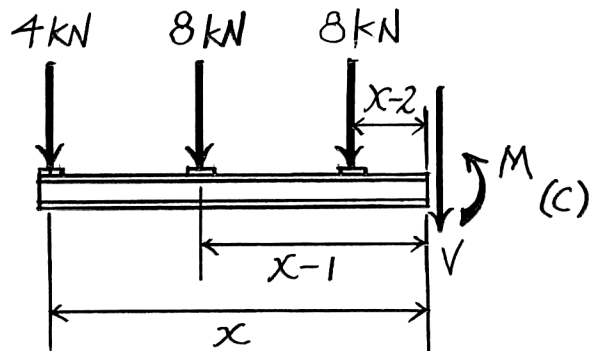
$$+\uparrow \sum F_y = 0; \quad -4 - 8 - 8 - V = 0 \quad V = \{-20\} \text{ kN}$$

Ans.

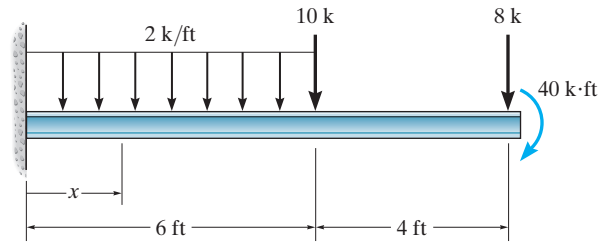
$$\zeta + \sum M_O = 0; \quad M + 4x + 8(x - 1) + 8(x - 2) = 0$$

$$M = \{-20x + 24\} \text{ kN}\cdot\text{m}$$

Ans.



4-18. Determine the shear and moment throughout the beam as functions of x .



Support Reactions: As shown on FBD.

Shear and Moment Functions:

For $0 \leq x < 6$ ft

$$+\uparrow \sum F_y = 0; \quad 30.0 - 2x - V = 0$$

$$V = \{30.0 - 2x\} \text{ k}$$

$$\zeta + \sum M_{NA} = 0; \quad M + 216 + 2x\left(\frac{x}{2}\right) - 30.0x = 0$$

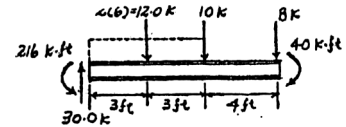
$$M = \{-x^2 + 30.0x - 216\} \text{ k} \cdot \text{ft}$$

For $6 \text{ ft} < x \leq 10$ ft

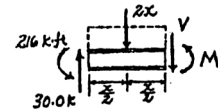
$$\rightarrow \sum F_y = 0; \quad V - 8 = 0 \quad V = 8.00 \text{ k}$$

$$\zeta + \sum M_{NA} = 0; \quad -M - 8(10 - x) - 40 = 0$$

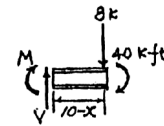
$$M = \{8.00x - 120\} \text{ k} \cdot \text{ft}$$



Ans.

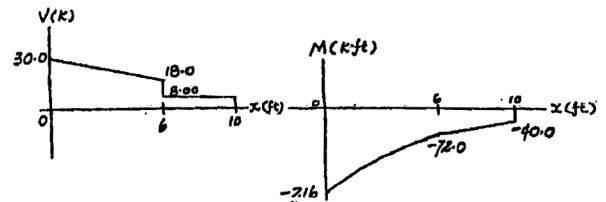


Ans.

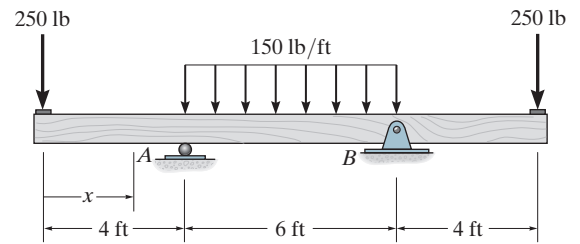


Ans.

Ans.



4-19. Determine the shear and moment throughout the beam as functions of x .



Support Reactions: As shown on FBD.

Shear and moment Functions:

For $0 \leq x < 4$ ft

$$+\uparrow \sum F_y = 0; \quad -250 - V = 0 \quad V = -250 \text{ lb}$$

$$\zeta + \sum M_{NA} = 0; \quad M + 250x = 0 \quad M = \{-250x\} \text{ lb} \cdot \text{ft}$$

For $4 \text{ ft} < x < 10$ ft

$$+\uparrow \sum F_y = 0; \quad -250 + 700 - 150(x - 4) - V = 0$$

$$V = \{1050 - 150x\} \text{ lb}$$

$$\zeta + \sum M_{NA} = 0; \quad M + 150(x - 4)\left(\frac{x - 4}{2}\right) + 250x - 700(x - 4) = 0$$

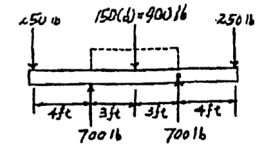
$$M = \{-75x^2 + 1050x - 4000\} \text{ lb} \cdot \text{ft}$$

For $10 \text{ ft} < x \leq 14$ ft

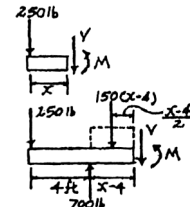
$$+\uparrow \sum F_y = 0; \quad V - 250 = 0 \quad V = 250 \text{ lb}$$

$$\zeta + \sum M_{NA} = 0; \quad -M - 250(14 - x) = 0$$

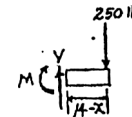
$$M = \{250x - 3500\} \text{ lb} \cdot \text{ft}$$



Ans.



Ans.

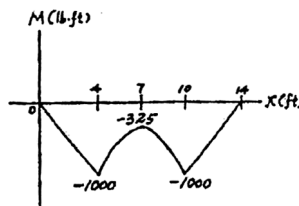
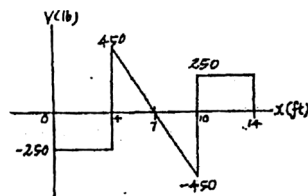


Ans.

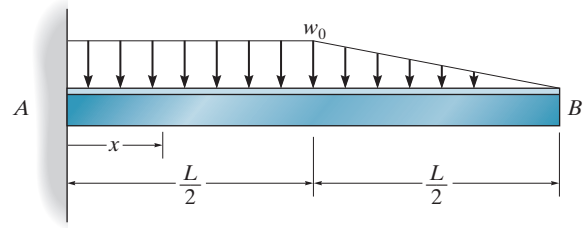
Ans.

Ans.

Ans.



*4-20. Determine the shear and moment in the beam as functions of x .



Support Reactions: As shown on FBD.

Shear and Moment Functions:

For $0 \leq x < L/2$

$$+\uparrow \sum F_y = 0; \quad \frac{3w_0L}{4} - w_0x - V = 0 \quad V = \frac{w_0}{4}(3L - 4x)$$

$$\zeta + \sum M_{NA} = 0; \quad \frac{7w_0L^2}{24} - \frac{3w_0L}{4}x + w_0x\left(\frac{x}{2}\right) + M = 0$$

$$M = \frac{w_0}{24}(-12x^2 + 18Lx - 7L^2)$$

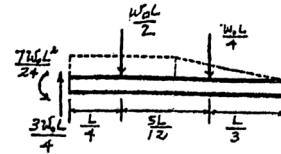
For $L/2 < x \leq L$

$$+\uparrow \sum F_y = 0; \quad V - \frac{1}{2}\left[\frac{2w_0}{L}(L-x)\right](L-x) = 0$$

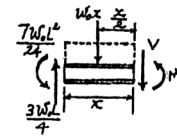
$$V = \frac{w_0}{L}(L-x)^2$$

$$\zeta + \sum M_{NA} = 0; \quad -M - \frac{1}{2}\left[\frac{2w_0}{L}(L-x)\right](L-x)\left(\frac{L-x}{3}\right) = 0$$

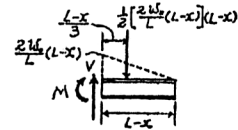
$$M = \frac{w_0}{3L}(L-x)^2$$



Ans.

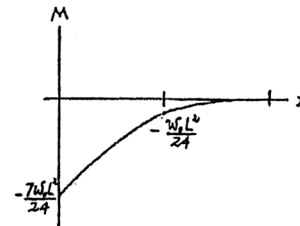
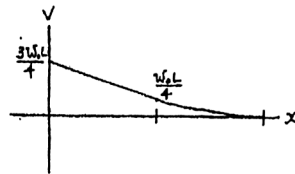


Ans.

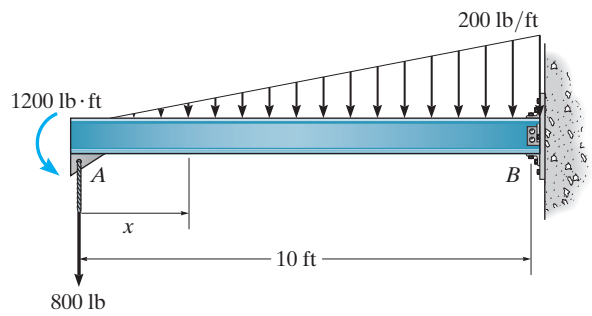


Ans.

Ans.



4-21. Determine the shear and moment in the beam as a function of x .



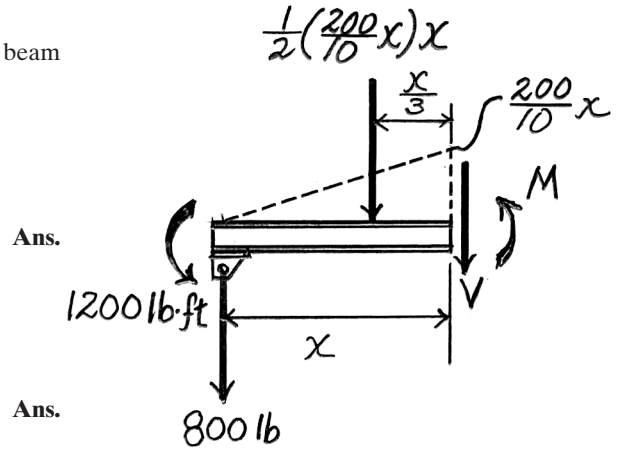
Internal Loadings: Referring to the FBD of the left segment of the beam in Fig. *a*,

$$+\uparrow \sum F_y = 0; \quad -800 - \frac{1}{2} \left(\frac{200}{10} x \right) (x) - V = 0$$

$$V = \{-10x^2 - 800\} \text{ lb}$$

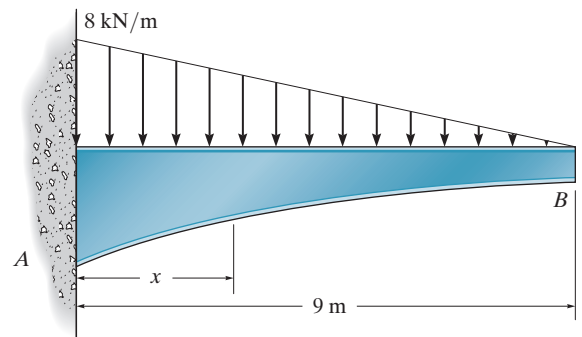
$$\zeta + \sum M_o = 0; \quad M + \frac{1}{2} \left(\frac{200}{10} x \right) (x) \left(\frac{x}{3} \right) + 800x + 1200 = 0$$

$$M = \{-3.33x^3 - 800x - 1200\} \text{ lb} \cdot \text{ft}$$



(a)

4-22 Determine the shear and moment throughout the tapered beam as a function of x .



$$\uparrow \sum F_y = 0; \quad 36 - \frac{1}{2} \left(\frac{8}{9} x \right) (x) - \frac{8}{9} \left(8 - \frac{8}{9} x \right) x - V = 0$$

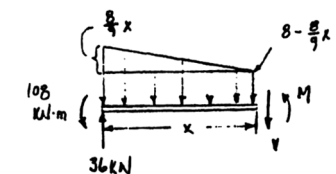
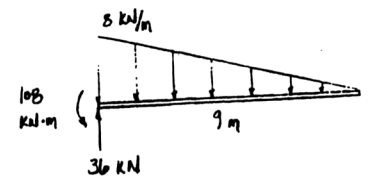
$$V = 36 - \frac{4}{9} x^2 - 8x + \frac{8}{9} x^2$$

$$V = 0.444x^2 - 8x + 36$$

$$\zeta + \sum M = 0; \quad 108 + \frac{1}{2} \left(\frac{8}{9} x \right) (x) \left(\frac{2}{3} x \right) + \frac{8}{9} \left(8 - \frac{8}{9} x \right) (x) \left(\frac{x}{2} \right) - 36x + M = 0$$

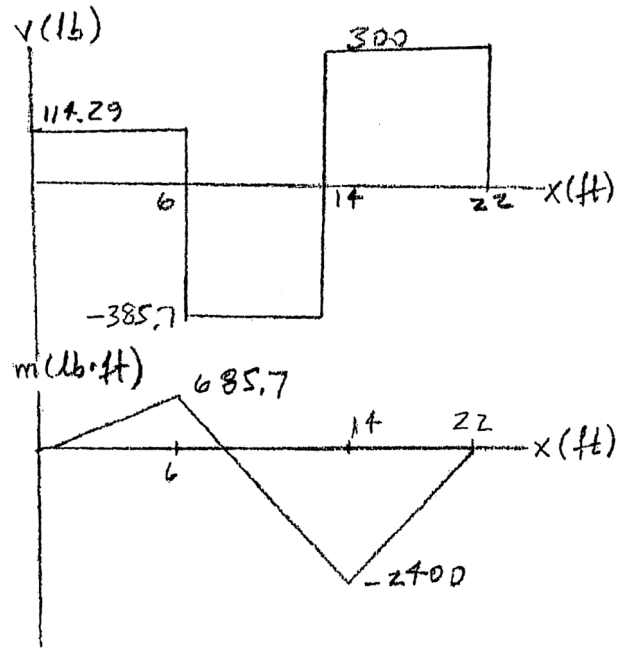
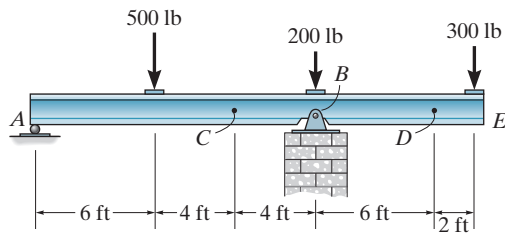
$$M = -108 - \frac{8}{27} x^3 - 4x^2 + \frac{8}{18} x^3 + 36x$$

$$M = 0.148x^3 - 4x^2 + 36x - 108$$

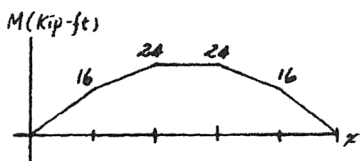
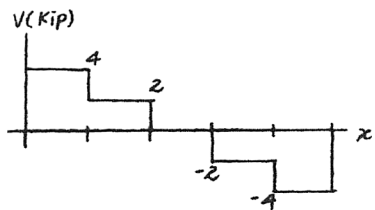
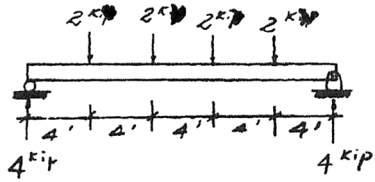
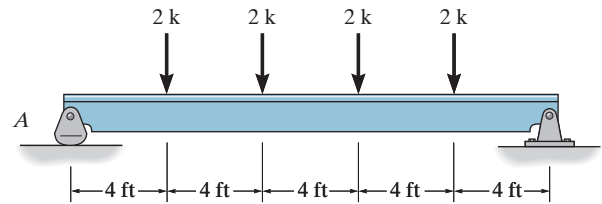


Ans.

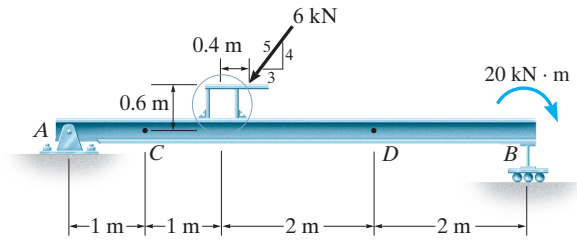
4-23. Draw the shear and moment diagrams for the beam.



*4-24. Draw the shear and moment diagrams for the beam.



4-25. Draw the shear and moment diagrams for the beam.

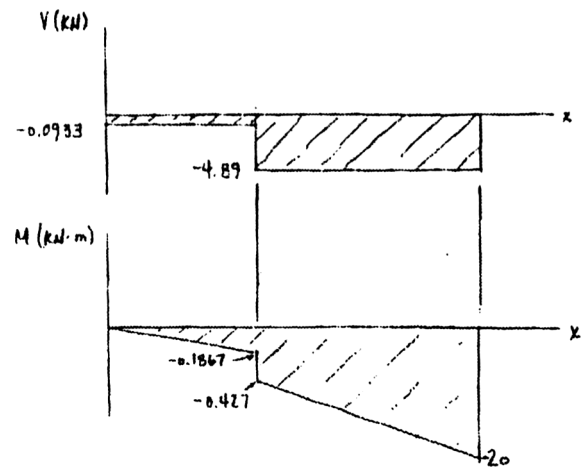
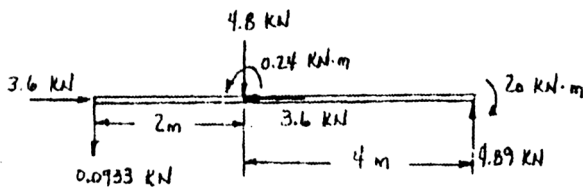


$$V_{\max} = -4.89 \text{ kN}$$

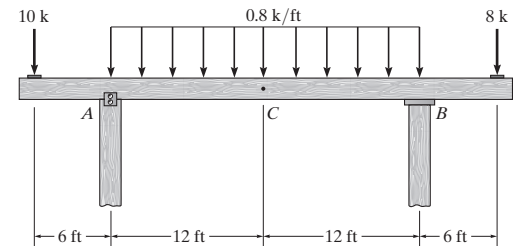
$$M_{\max} = -20 \text{ kN}\cdot\text{m}$$

Ans.

Ans.



4-26. Draw the shear and moment diagrams of the beam.

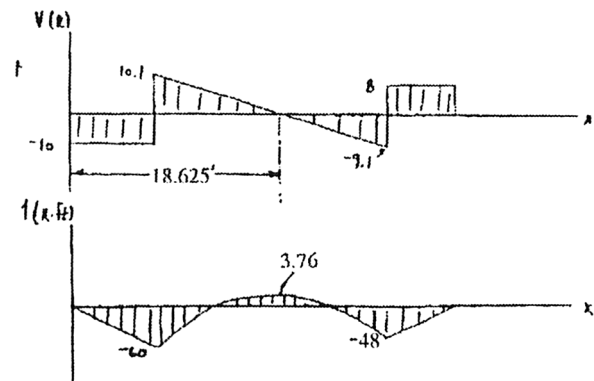
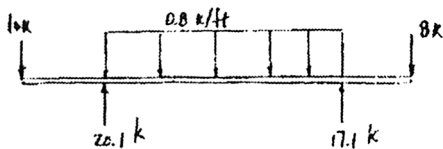


$$V_{\max} = -10.1 \text{ k}$$

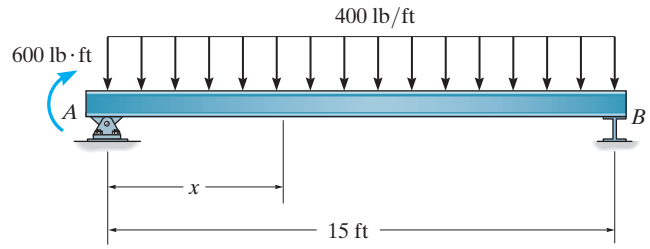
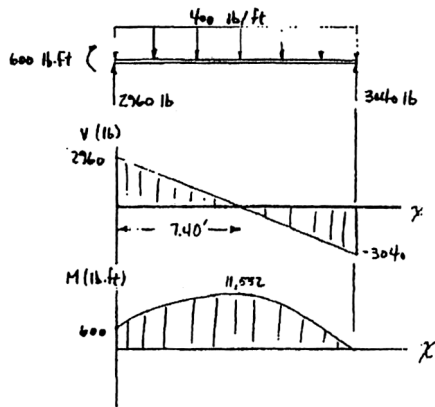
$$M_{\max} = -60 \text{ k}\cdot\text{ft}$$

Ans.

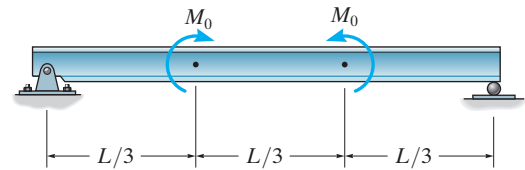
Ans.

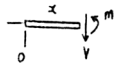


4-27. Draw the shear and moment diagrams for the beam.



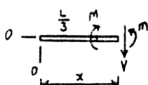
*4-28. Draw the shear and moment diagrams for the beam (a) in terms of the parameters shown; (b) set $M_O = 500 \text{ N} \cdot \text{m}$, $L = 8 \text{ m}$.



(a) For $0 \leq x \leq \frac{L}{3}$ 


$$+\uparrow \sum F_y = 0; \quad V = 0$$

$$\zeta + \sum M = 0; \quad M = 0$$

For $\frac{L}{3} < x < \frac{2L}{3}$ 

$$+\uparrow \sum F_y = 0; \quad V = 0$$

$$\zeta + \sum M = 0; \quad M = M_O$$

For $\frac{2L}{3} < x \leq L$ 

$$+\uparrow \sum F_y = 0; \quad V = 0$$

$$\zeta + \sum M = 0; \quad M = 0$$

Ans.

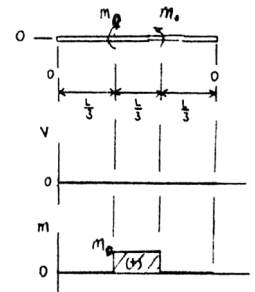
Ans.

Ans.

Ans.

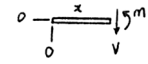
Ans.

Ans.



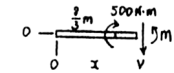
4-28. Continued

(b) Set $M_O = 500 \text{ N} \cdot \text{m}$, $L = 8 \text{ m}$

For $0 \leq x < \frac{8}{3} \text{ m}$ 

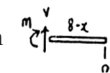
$$+\uparrow \sum F_y = 0; \quad V = 0$$

$$\zeta + \sum M = 0; \quad M = 0$$

For $\frac{8}{3} \text{ m} < x < \frac{16}{3} \text{ m}$ 

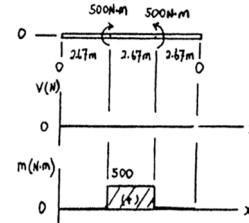
$$+\uparrow \sum F_y = 0; \quad V = 0$$

$$\zeta + \sum M = 0; \quad M = 500 \text{ N} \cdot \text{m}$$

For $\frac{16}{3} \text{ m} < x \leq 8 \text{ m}$ 

$$+\uparrow \sum F_y = 0; \quad V = 0$$

$$\zeta + \sum M = 0; \quad M = 0$$



Ans.

Ans.

Ans.

Ans.

Ans.

Ans.

4-29. Draw the shear and moment diagrams for the beam.

Support Reactions:

$$\zeta + \sum M_A = 0; \quad C_x(3) - 1.5(2.5) = 0 \quad C_x = 1.25 \text{ kN}$$

$$+\uparrow \sum F_y = 0; \quad A_y - 1.5 + 1.25 = 0 \quad A_y = 0.250 \text{ kN}$$

Shear and Moment Functions: For $0 \leq x < 2 \text{ m}$ [FBD (a)],

$$+\uparrow \sum F_y = 0; \quad 0.250 - V = 0 \quad V = 0.250 \text{ kN}$$

$$\zeta + \sum M = 0; \quad M - 0.250x = 0 \quad M = (0.250x) \text{ kN} \cdot \text{m}$$

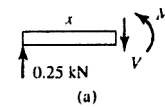
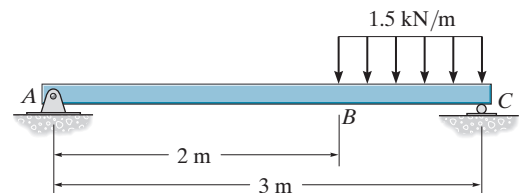
For $2 \text{ m} < x \leq 3 \text{ m}$ [FBD (b)],

$$+\uparrow \sum F_y = 0; \quad 0.25 - 1.5(x - 2) - V = 0$$

$$V = (3.25 - 1.50x) \text{ kN}$$

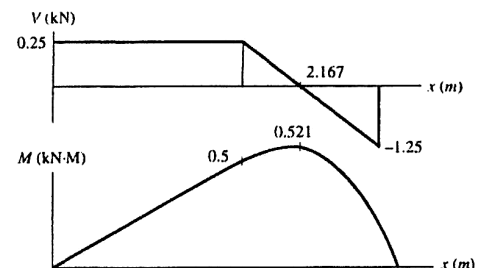
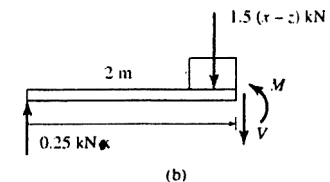
$$\zeta + \sum M = 0; \quad -0.25x + 1.5(x + 2) \left(\frac{x - 2}{2} \right) + M = 0$$

$$M = (-0.750x^2 + 3.25x - 3.00) \text{ kN} \cdot \text{m}$$



Ans.

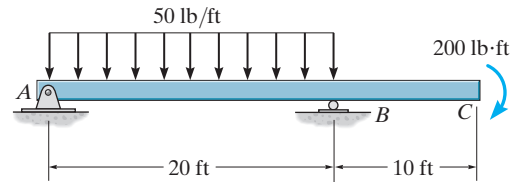
Ans.



Ans.

Ans.

4-30. Draw the shear and bending-moment diagrams for the beam.



Support Reactions:

$$\zeta + \sum M_B = 0; \quad 1000(10) - 200 - A_y(20) = 0 \quad A_y = 490 \text{ lb}$$

Shear and Moment Functions: For $0 \leq x < 20 \text{ ft}$ [FBD (a)].

$$+\uparrow \sum F_y = 0; \quad 490 - 50x - V = 0$$

$$V = \{490 - 50.0x\} \text{ lb}$$

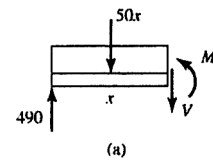
$$\zeta + \sum M = 0; \quad M + 50x\left(\frac{x}{2}\right) - 490x = 0$$

$$M = (490x - 25.0x^2) \text{ lb} \cdot \text{ft}$$

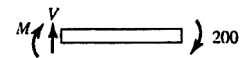
For $20 \text{ ft} < x \leq 30 \text{ ft}$ [FBD (b)],

$$+\uparrow \sum F_y = 0; \quad V = 0$$

$$\zeta + \sum M = 0; \quad -200 - M = 0 \quad M = -200 \text{ lb} \cdot \text{ft}$$

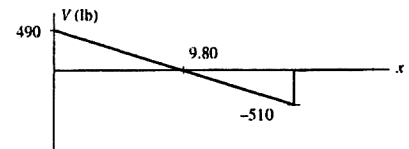


Ans.

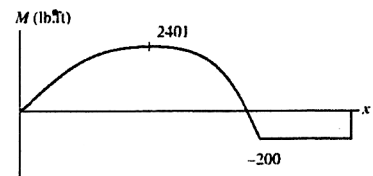


Ans.

Ans.

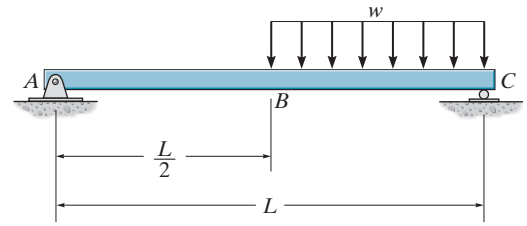


Ans.



(b)

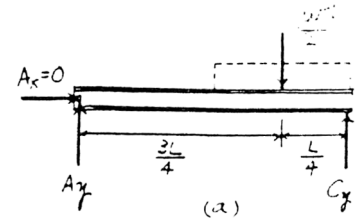
4-31. Draw the shear and moment diagrams for the beam.



Support Reactions: From FBD(a),

$$\zeta + \sum M_A = 0; \quad C_y(L) - \frac{wL}{2} \left(\frac{3L}{4} \right) = 0 \quad C_y = \frac{3wL}{8}$$

$$+\uparrow \sum F_y = 0; \quad A_y + \frac{3wL}{8} - \left(\frac{wL}{2} \right) = 0 \quad A_y = \frac{wL}{8}$$

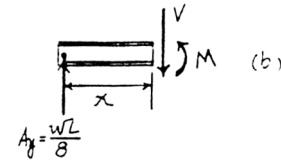


Shear and Moment Functions: For $0 \leq x < \frac{L}{2}$ [FBD (b)],

$$+\uparrow \sum F_y = 0; \quad \frac{wL}{8} - V = 0 \quad V = \frac{wL}{8}$$

$$\zeta + \sum M = 0; \quad M - \frac{wL}{8}(x) = 0 \quad M = \frac{wL}{8}(x)$$

Ans.



Ans.

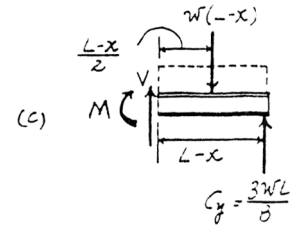
For $\frac{L}{2} < x \leq L$ [FBD (c)],

$$+\uparrow \sum F_y = 0; \quad V + \frac{3wL}{8} - w(L-x) = 0$$

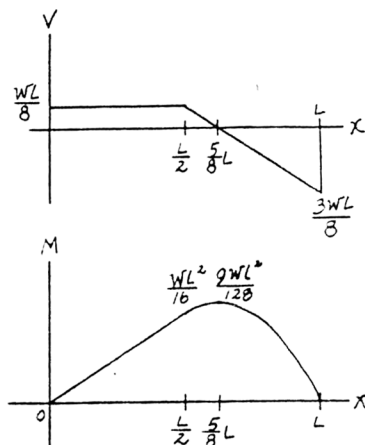
$$V = \frac{w}{8}(5L - 8x)$$

$$\zeta + \sum M_B = 0; \quad \frac{3wL}{8}(L-x) - w(L-x) \left(\frac{L-x}{2} \right) - M = 0$$

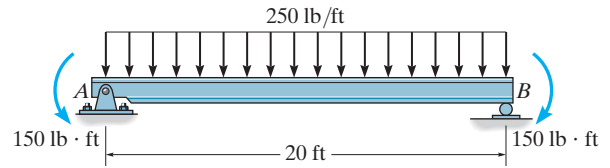
$$M = \frac{w}{8}(-L^2 + 5Lx - 4x^2)$$



Ans.



*4-32. Draw the shear and moment diagrams for the beam.



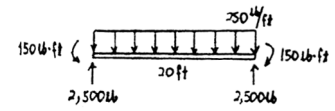
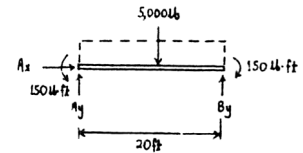
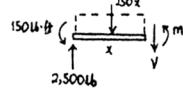
$$\zeta + \sum M_A = 0; \quad -5000(10) - 150 + B_y(20) = 0$$

$$B_y = 2500 \text{ lb}$$

$$\rightarrow \sum F_x = 0; \quad A_x = 0$$

$$+\uparrow \sum F_y = 0; \quad A_y + 2500 - 5000 = 0$$

$$A_y = 2500 \text{ lb}$$



For $0 \leq x \leq 20 \text{ ft}$

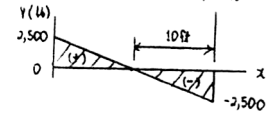
$$+\uparrow \sum F_y = 0; \quad 2500 - 250x - V = 0$$

$$V = 250(10 - x)$$

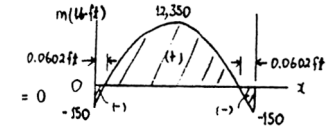
$$\zeta + \sum M = 0; \quad -2500(x) + 150 + 250x\left(\frac{x}{2}\right) + M = 0$$

$$M = 25(100x - 5x^2 - 6)$$

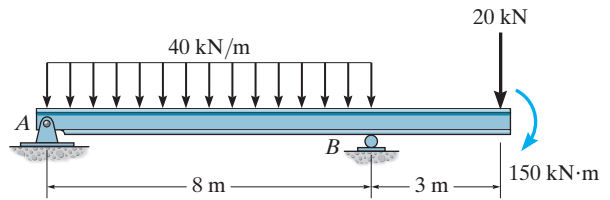
Ans.



Ans.



4-33. Draw the shear and moment diagrams for the beam.



$0 \leq x < 8$

$$+\uparrow \sum F_y = 0; \quad 133.75 - 40x - V = 0$$

$$V = 133.75 - 40x$$

$$\zeta + \sum M = 0; \quad M + 40x\left(\frac{x}{2}\right) - 133.75x = 0$$

$$M = 133.75x - 20x^2$$

$8 < x \leq 11$

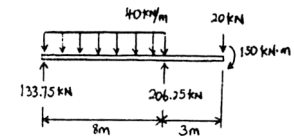
$$+\uparrow \sum F_y = 0; \quad V - 20 = 0$$

$$V = 20$$

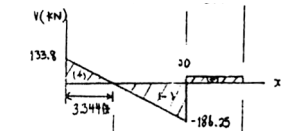
$$\zeta + \sum M = 0; \quad M + 20(11 - x) + 150 = 0$$

$$M = 20x - 370$$

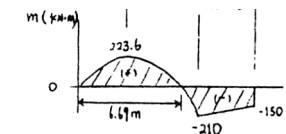
Ans.



Ans.

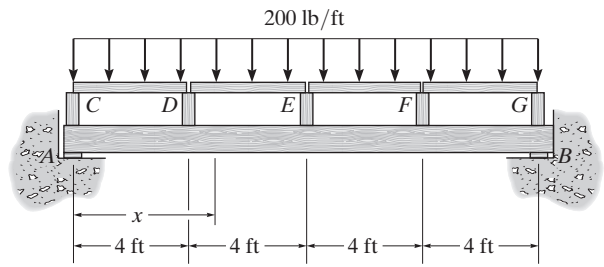


Ans.



Ans.

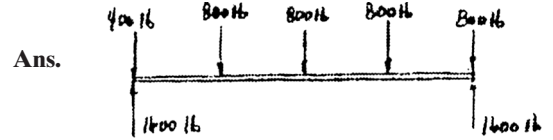
4-34. Draw the shear and moment diagrams for the beam.



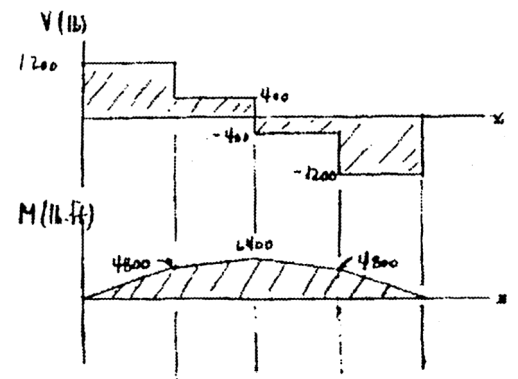
$$V_{\max} = \pm 1200 \text{ lb}$$

$$M_{\max} = 6400 \text{ lb} \cdot \text{ft}$$

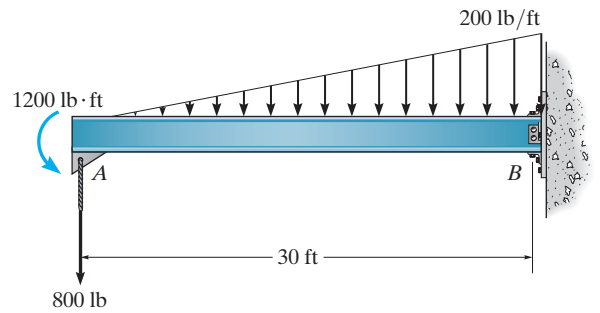
Ans.



Ans.



4-35. Draw the shear and moment diagrams for the beam.

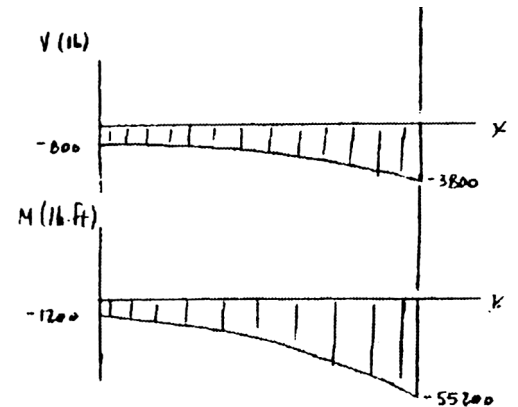
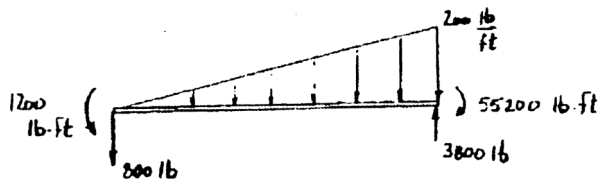


$$V_{\max} = -3.80 \text{ k}$$

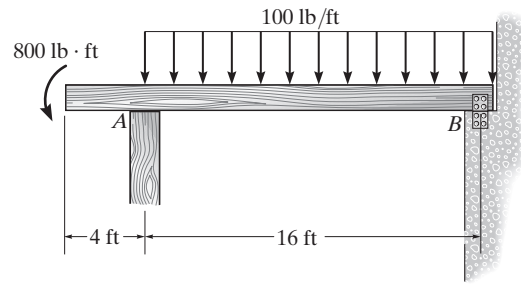
$$M_{\max} = -55.2 \text{ k} \cdot \text{ft}$$

Ans.

Ans.



*4-36. Draw the shear and moment diagrams of the beam. Assume the support at B is a pin and A is a roller.

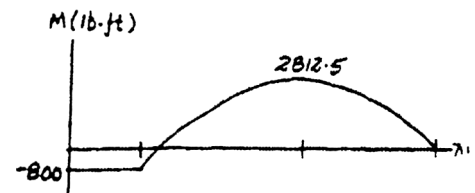
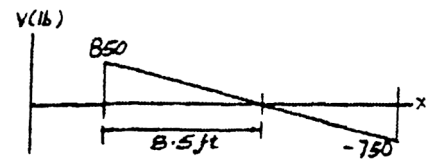
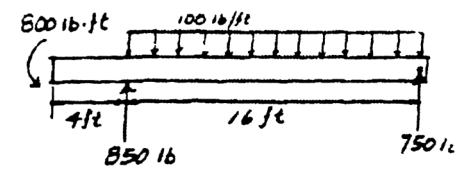


$V_{\max} = 850 \text{ lb}$

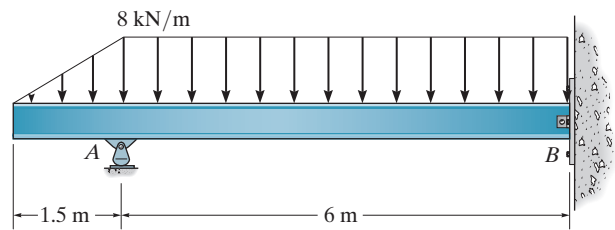
$M_{\max} = -2.81 \text{ K} \cdot \text{ft}$

Ans.

Ans.



4-37. Draw the shear and moment diagrams for the beam. Assume the support at B is a pin.

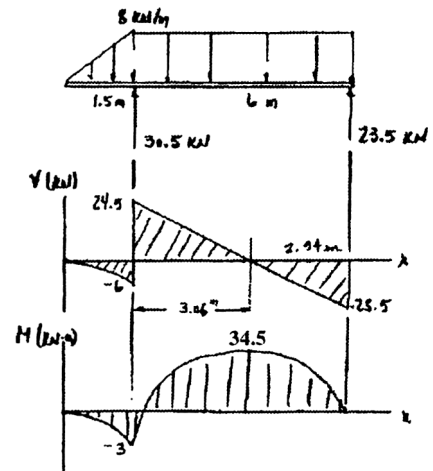


$V_{\max} = 24.5 \text{ kN}$

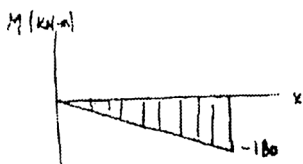
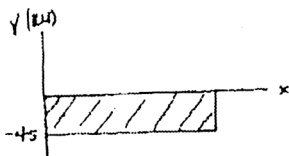
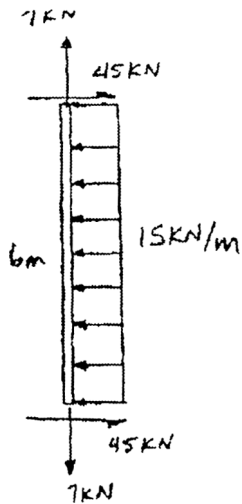
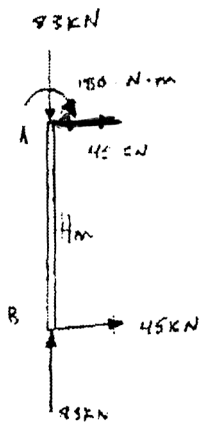
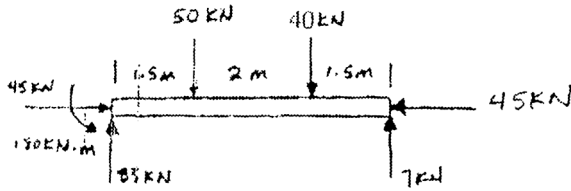
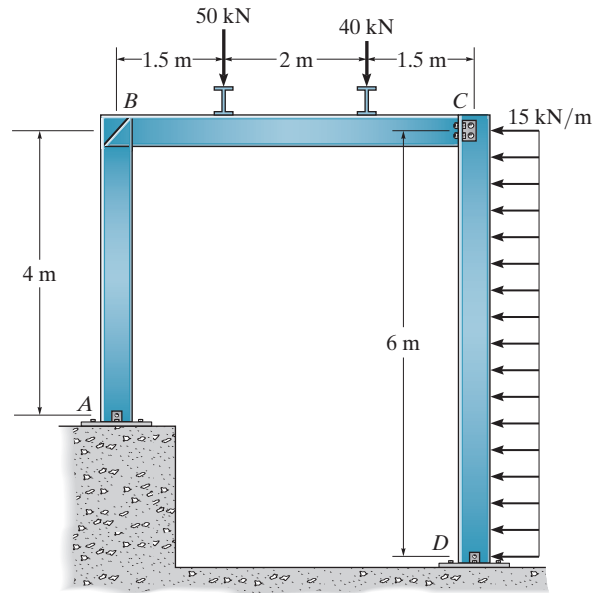
$M_{\max} = 34.5 \text{ kN} \cdot \text{m}$

Ans.

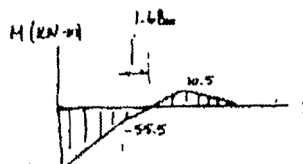
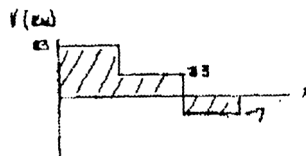
Ans.



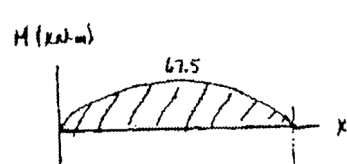
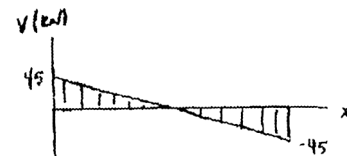
4-38. Draw the shear and moment diagrams for each of the three members of the frame. Assume the frame is pin connected at A, C, and D and there is fixed joint at B.



AB

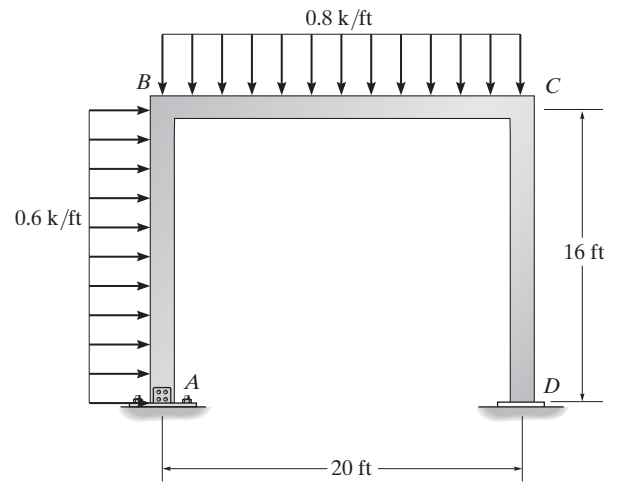


BC



CD

4-39. Draw the shear and moment diagrams for each member of the frame. Assume the support at *A* is a pin and *D* is a roller.

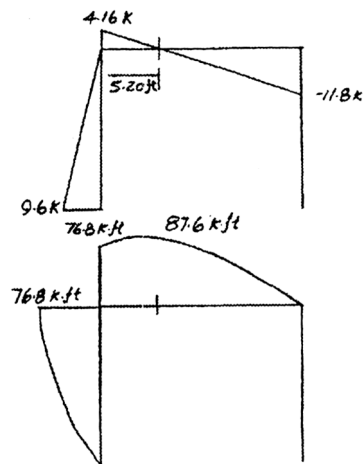
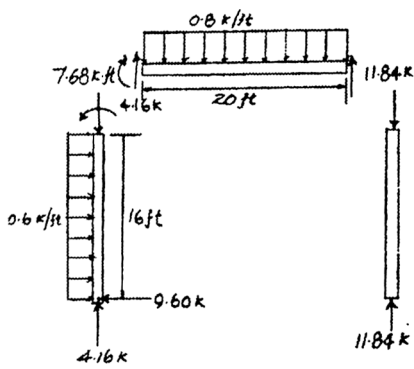


$$V_{\max} = -11.8 \text{ k}$$

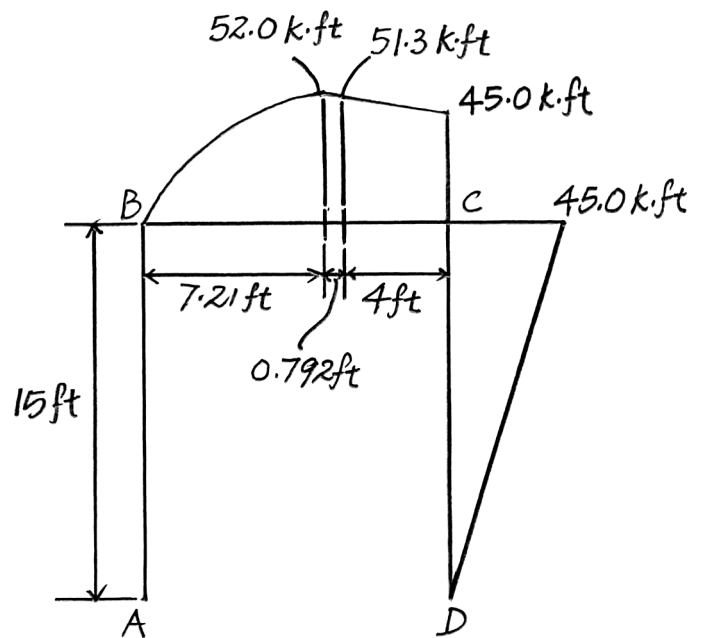
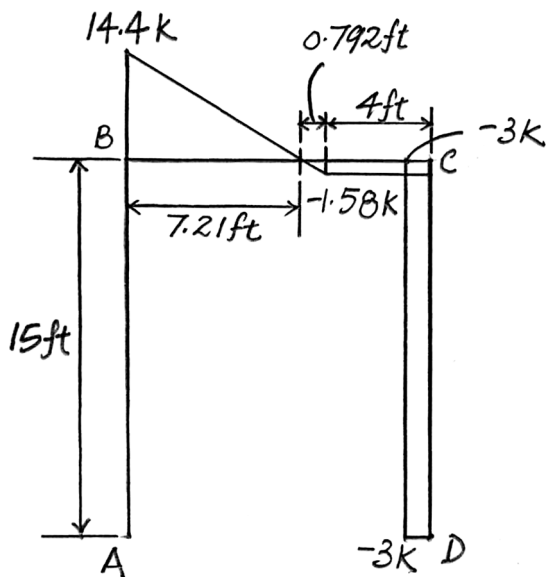
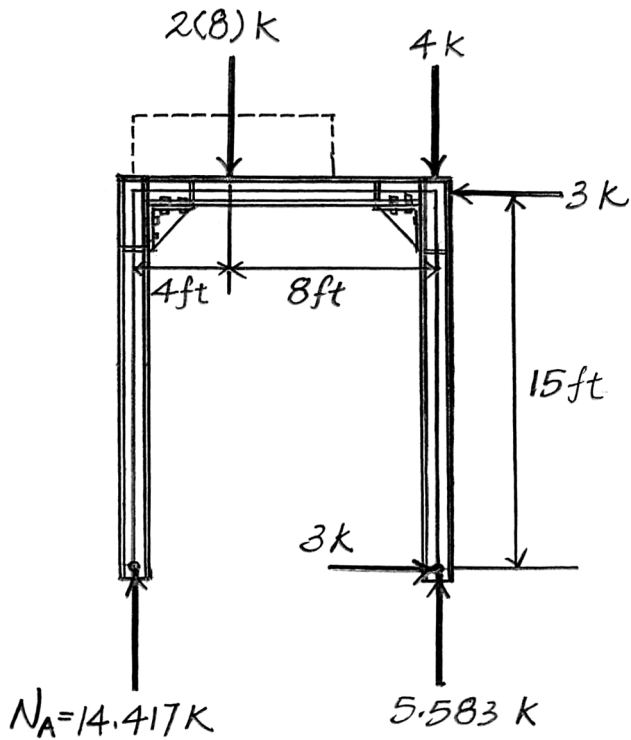
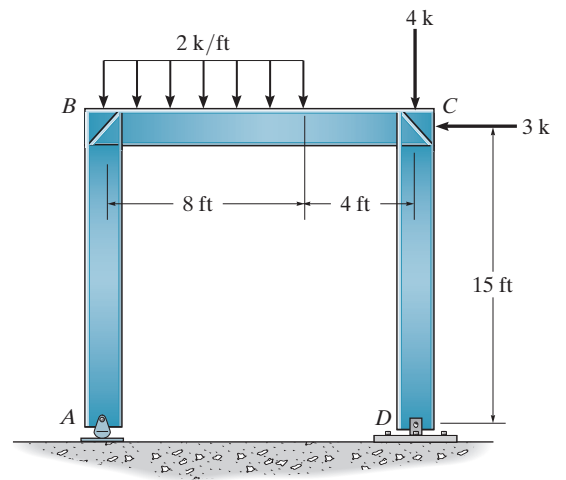
$$M_{\max} = -87.6 \text{ k} \cdot \text{ft}$$

Ans.

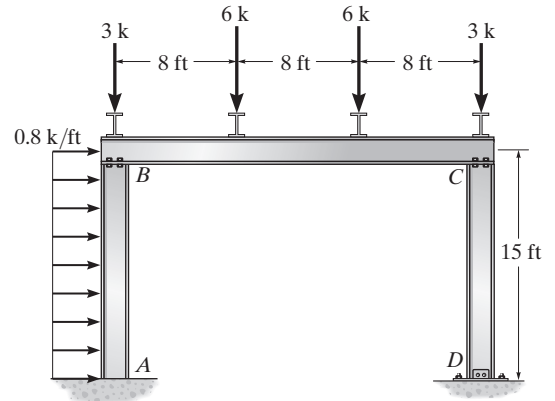
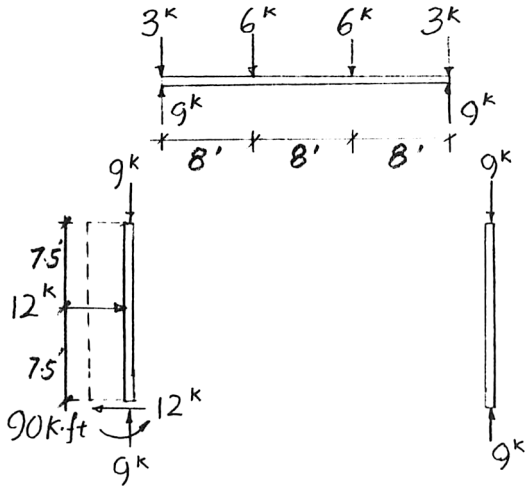
Ans.



*4-40. Draw the shear and moment diagrams for each member of the frame. Assume A is a rocker, and D is pinned.

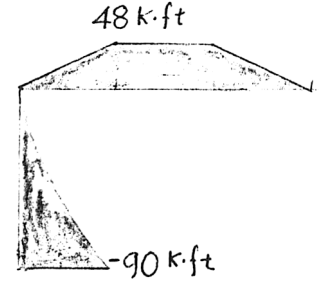
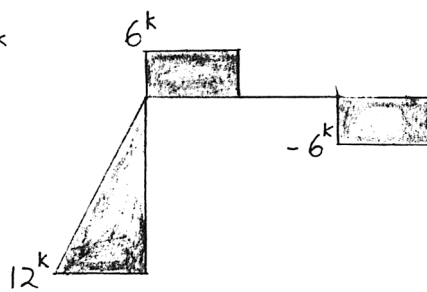


4-41. Draw the shear and moment diagrams for each member of the frame. Assume the frame is pin connected at B, C, and D and A is fixed.



Shear diagram

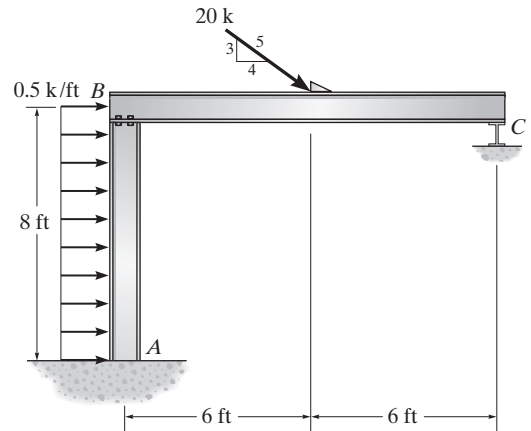
moment diagram



4-42. Draw the shear and moment diagrams for each member of the frame. Assume A is fixed, the joint at B is a pin, and support C is a roller.

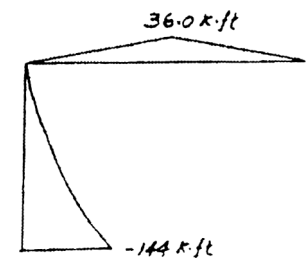
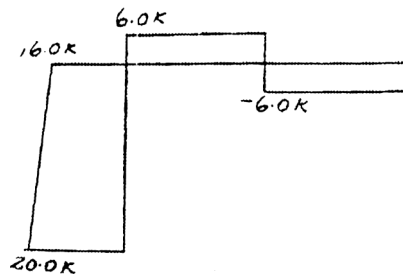
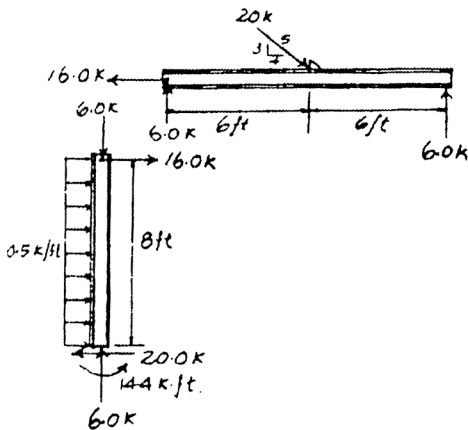
$V_{\max} = 20.0 \text{ k}$

$M_{\max} = -144 \text{ k} \cdot \text{ft}$

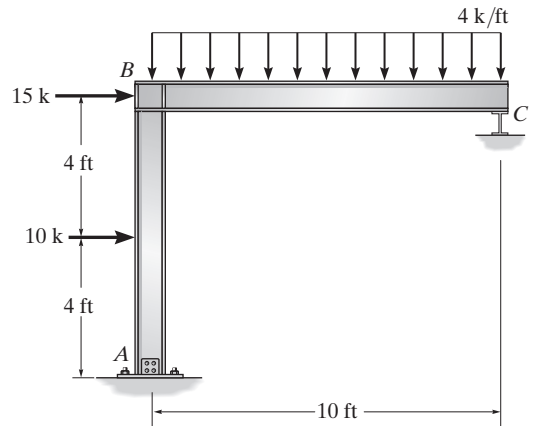


Ans.

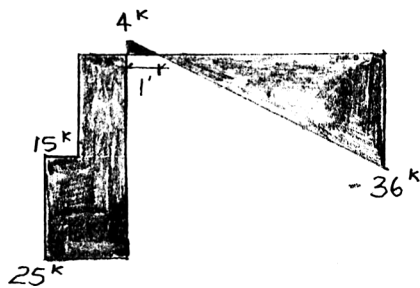
Ans.



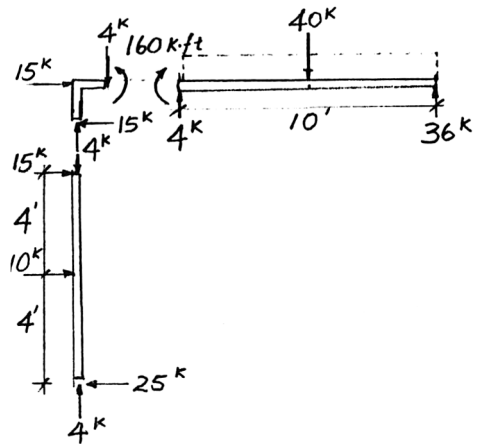
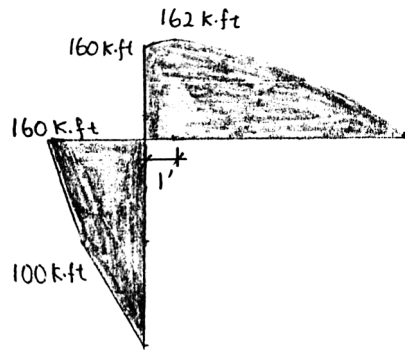
4-43. Draw the shear and moment diagrams for each member of the frame. Assume the frame is pin connected at A, and C is a roller.



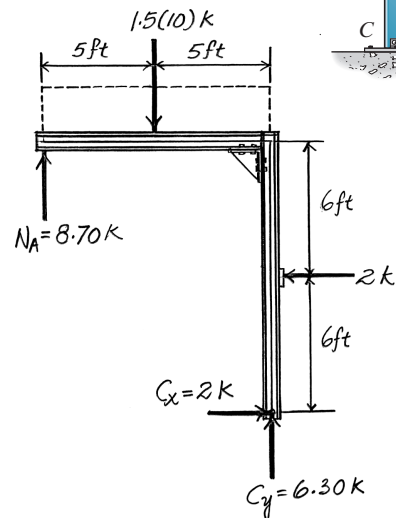
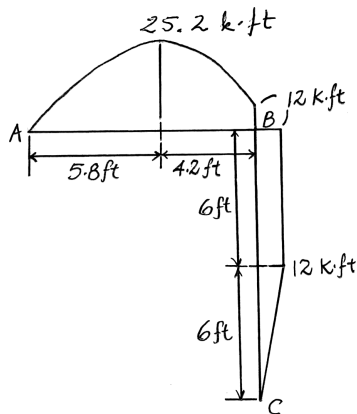
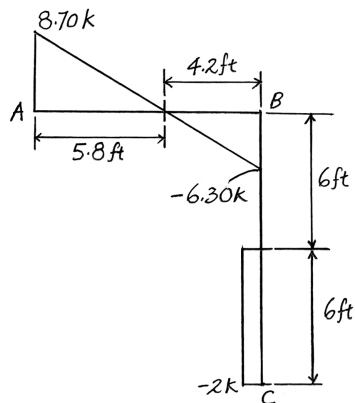
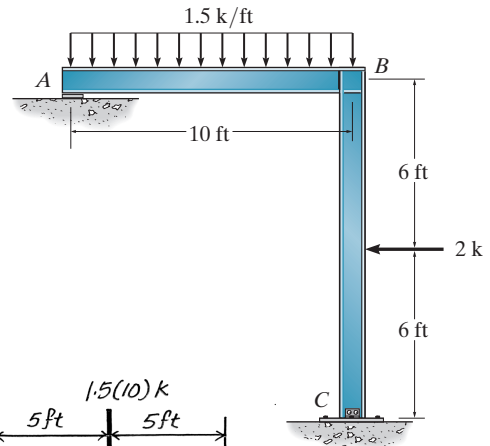
Shear diagram



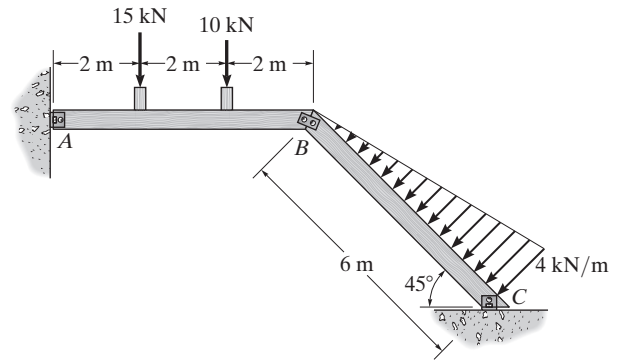
moment diagram



*4-44. Draw the shear and moment diagrams for each member of the frame. Assume the frame is roller supported at A and pin supported at C.



4-45. Draw the shear and moment diagrams for each member of the frame. The members are pin connected at A, B, and C.



Support Reactions:

$$\zeta + \sum M_A = 0; \quad -15(2) - 10(4) + B_y(6) = 0$$

$$B_y = 11.667 \text{ kN}$$

$$+\uparrow \sum F_y = 0; \quad A_y - 25 + 11.667 = 0$$

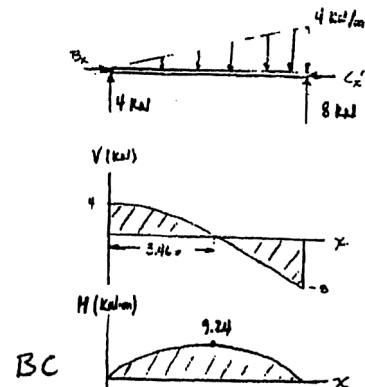
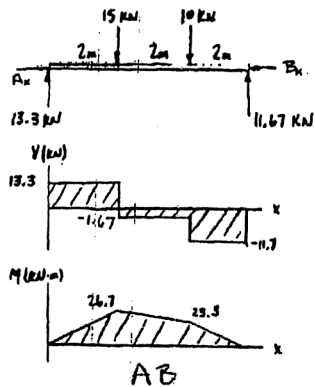
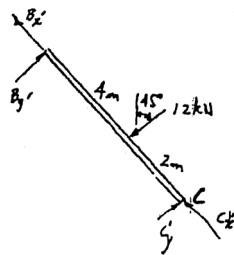
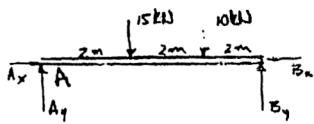
$$A_y = 13.3 \text{ kN}$$

$$\zeta + \sum M_C = 0; \quad 12(2) - B_{y'}(6) = 0$$

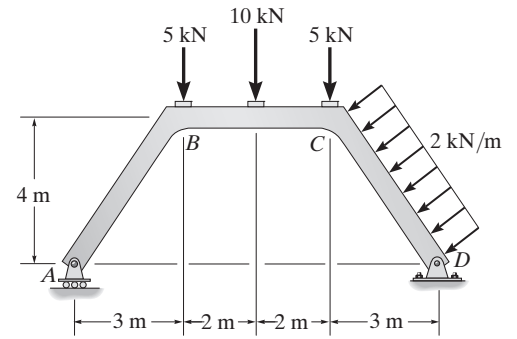
$$B_{y'} = 4 \text{ kN}$$

$$+\nearrow \sum F_{y'} = 0; \quad 4 - 12 + C_{y'} = 0$$

$$C_{y'} = 8 \text{ kN}$$



4-46. Draw the shear and moment diagrams for each member of the frame.



$$\zeta + \sum M_D = 0; \quad 10(2.5) + 5(3) + 10(5) + 5(7) - A_y(10) = 0$$

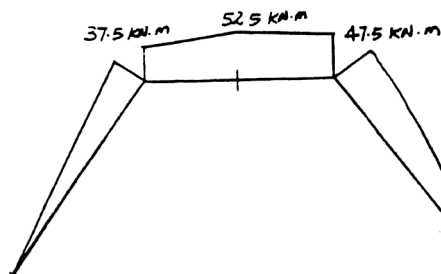
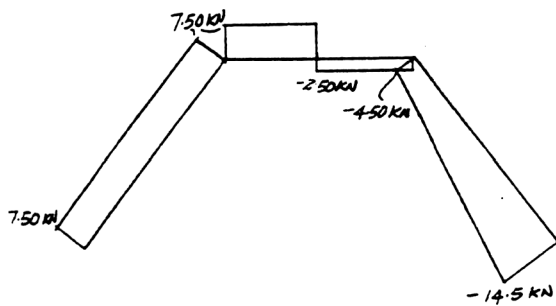
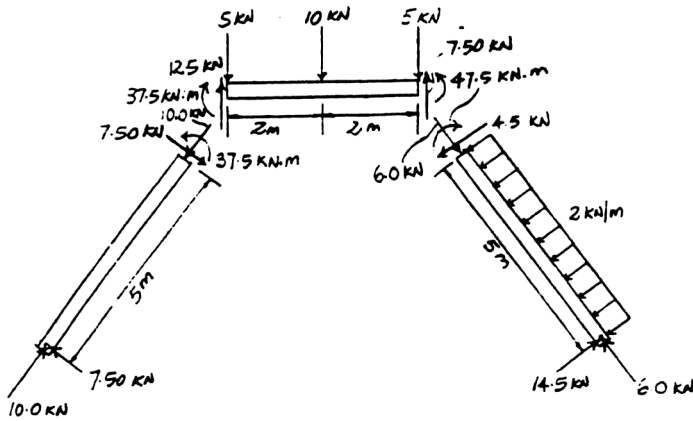
$$A_y = 12.5 \text{ kN}$$

$$\rightarrow \sum F_x = 0; \quad -10\left(\frac{4}{5}\right) + D_x = 0$$

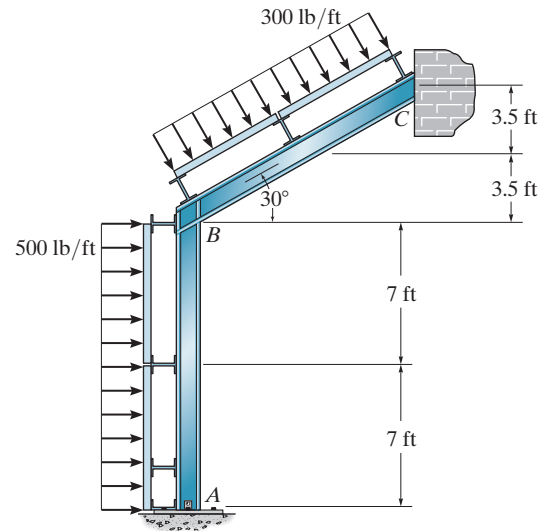
$$D_x = 8 \text{ kN}$$

$$+\uparrow \sum F_y = 0; \quad 12.5 - 5 - 10 - 5 - 10\left(\frac{3}{5}\right) + D_y = 0$$

$$D_y = 13.5 \text{ kN}$$



4-47. Draw the shear and moment diagrams for each member of the frame. Assume the joint at *A* is a pin and support *C* is a roller. The joint at *B* is fixed. The wind load is transferred to the members at the girts and purlins from the simply supported wall and roof segments.



Support Reactions:

$$\zeta + \sum M_A = 0; \quad -3.5(7) - 1.75(14) - (4.20)(\sin 30^\circ)(7 \cos 30^\circ) - 4.20(\sin 30^\circ)(14 + 3.5) + (21) = 0$$

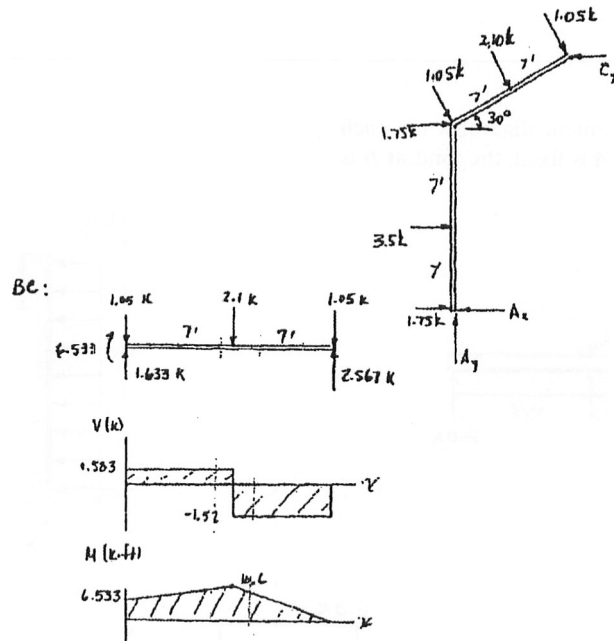
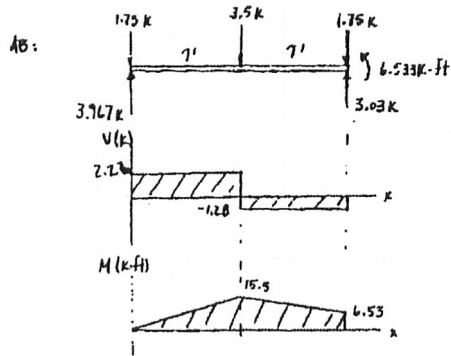
$$C_x = 5.133 \text{ kN}$$

$$\rightarrow \sum F_x = 0; \quad 1.75 + 3.5 + 1.75 + 4.20 \sin 30^\circ - 5.133 - A_x = 0$$

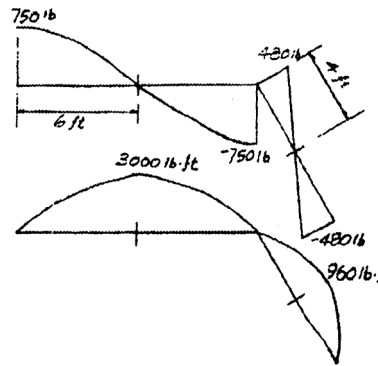
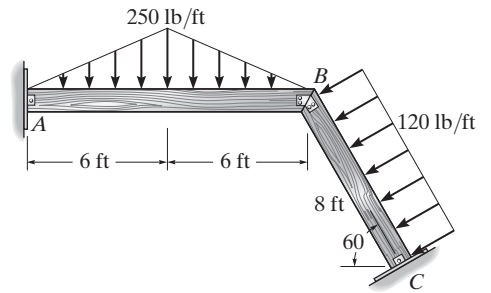
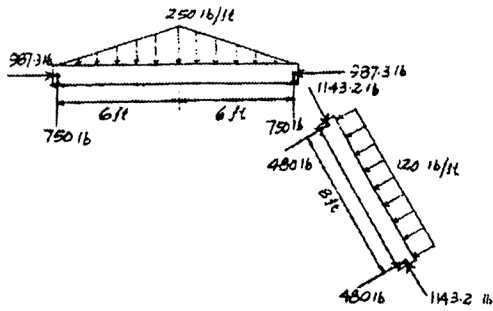
$$A_x = 3.967 \text{ kN}$$

$$+\uparrow \sum F_y = 0; \quad A_y - 4.20 \cos 30^\circ = 0$$

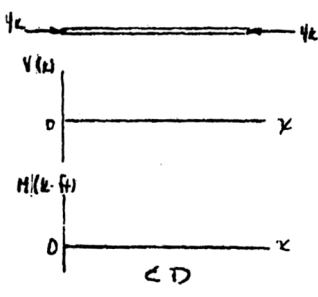
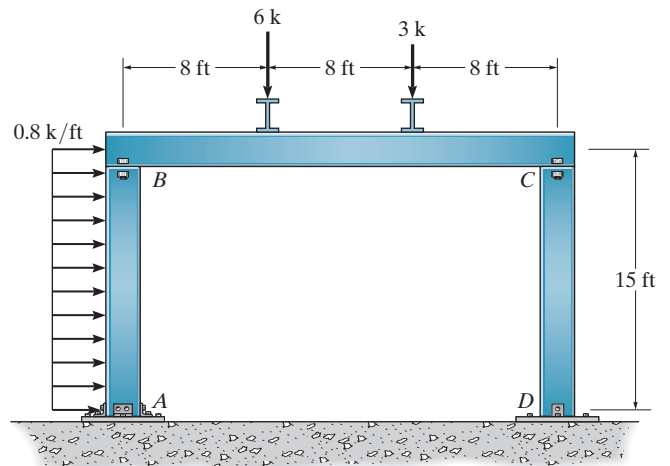
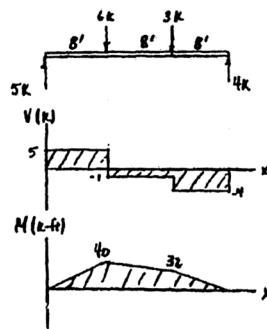
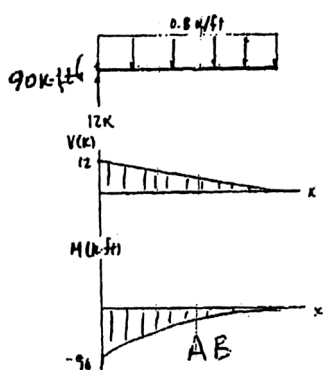
$$A_y = 3.64 \text{ kN}$$



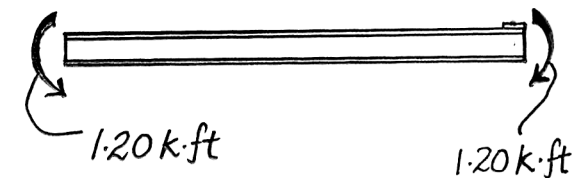
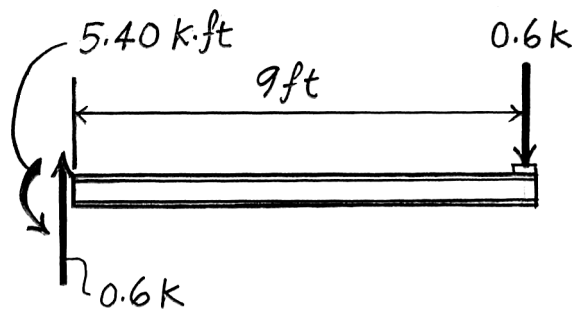
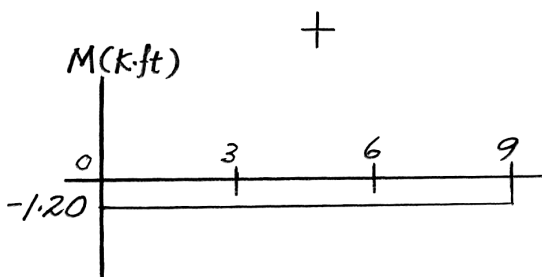
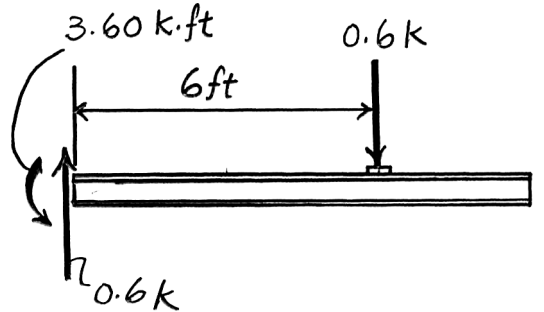
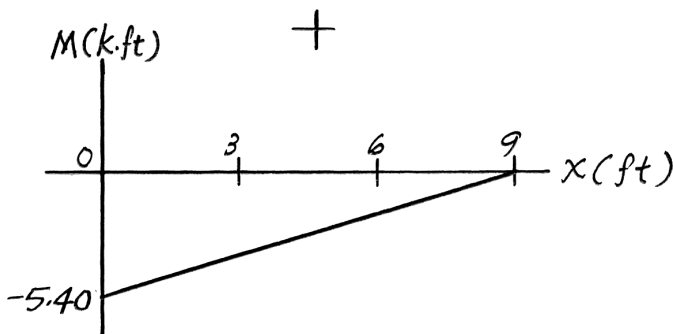
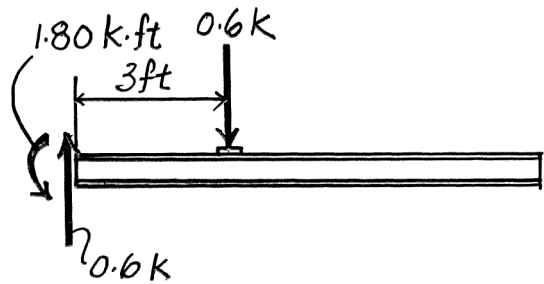
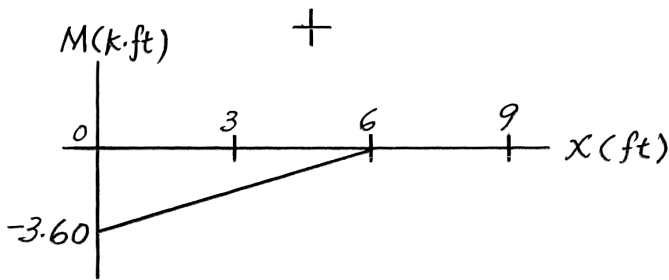
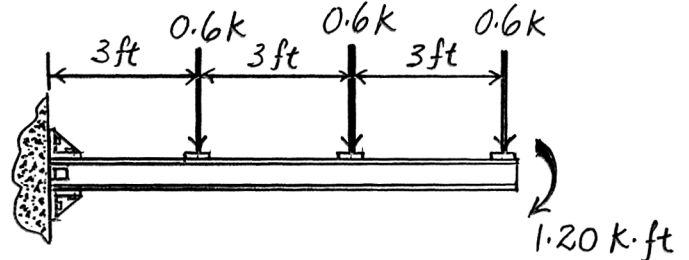
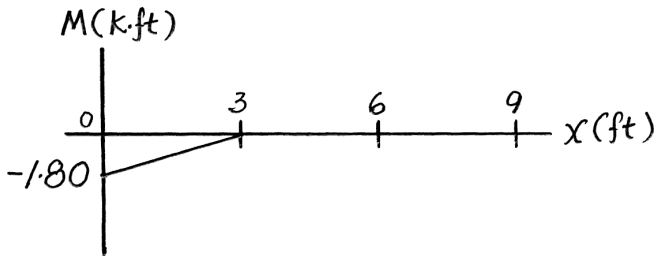
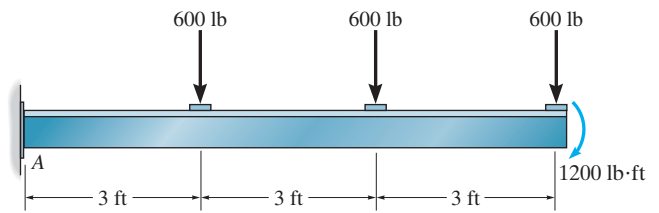
*4-48. Draw the shear and moment diagrams for each member of the frame. The joints at A, B and C are pin connected.



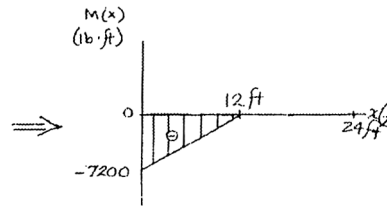
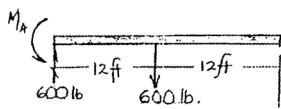
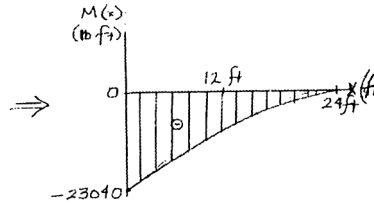
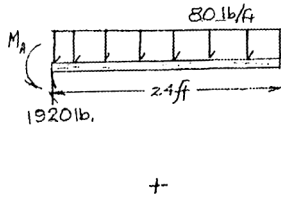
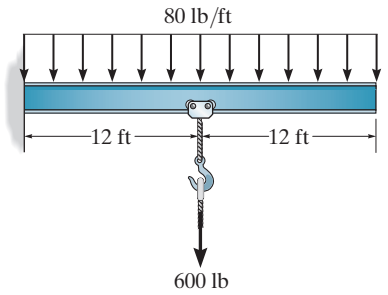
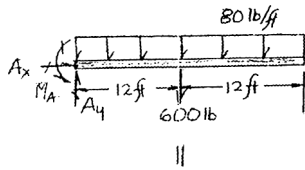
4-49. Draw the shear and moment diagrams for each of the three members of the frame. Assume the frame is pin connected at B, C and D and A is fixed.



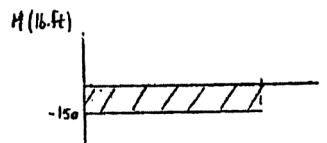
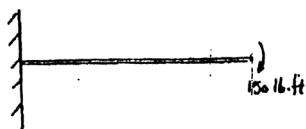
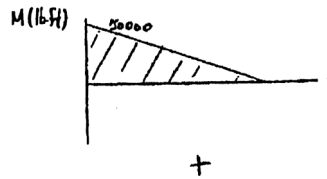
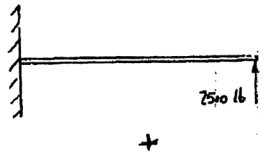
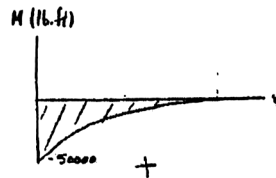
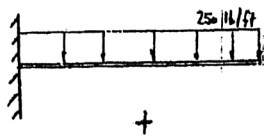
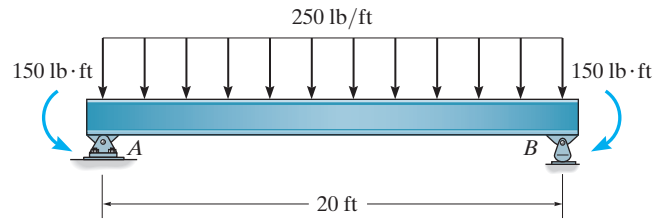
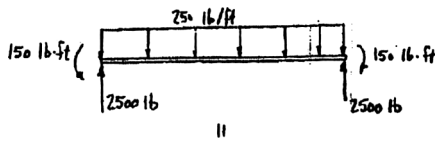
4-50. Draw the moment diagrams for the beam using the method of superposition. The beam is cantilevered from A.



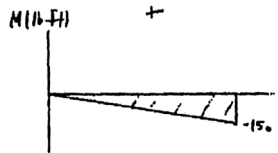
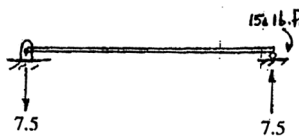
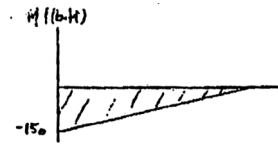
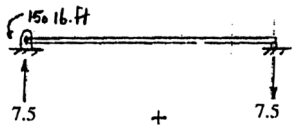
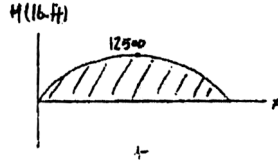
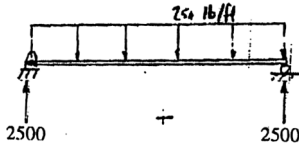
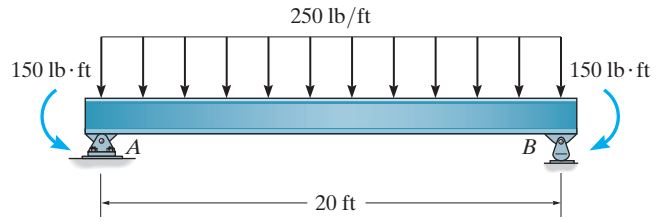
4-51. Draw the moment diagrams for the beam using the method of superposition.



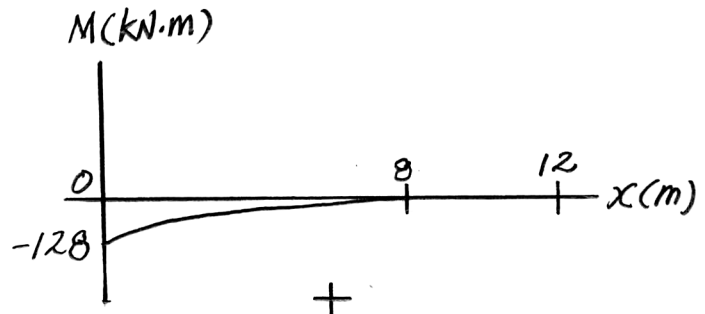
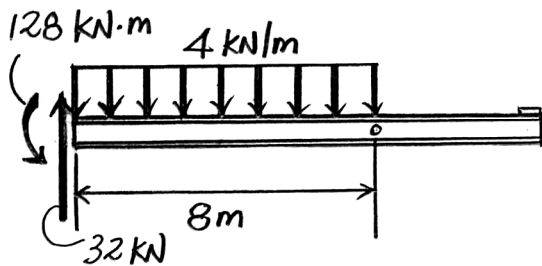
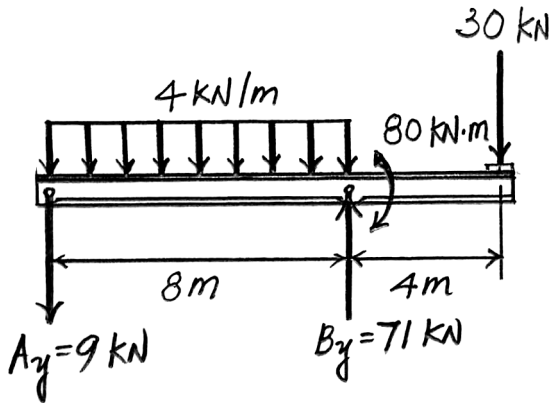
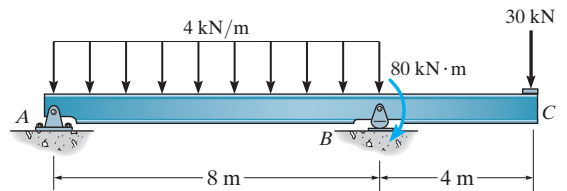
***4-52.** Draw the moment diagrams for the beam using the method of superposition. Consider the beam to be cantilevered from end A.



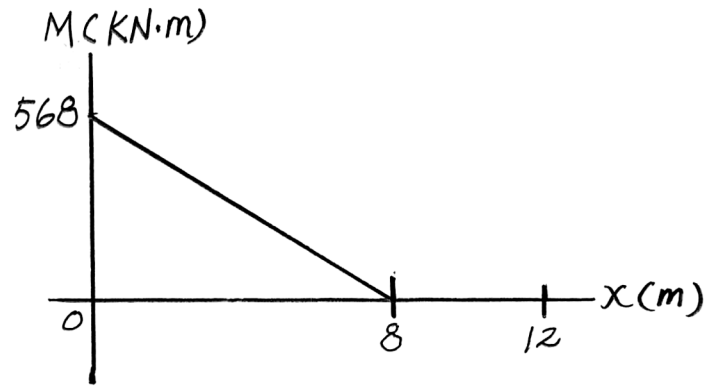
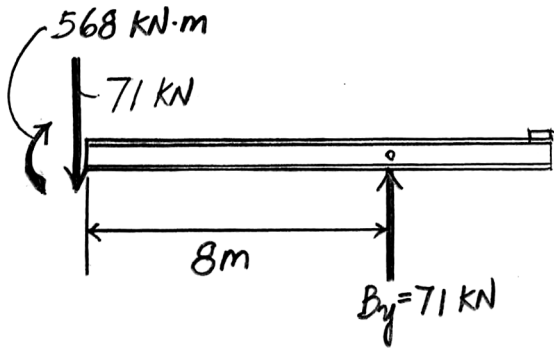
4-53. Draw the moment diagrams for the beam using the method of superposition. Consider the beam to be simply supported at A and B as shown.



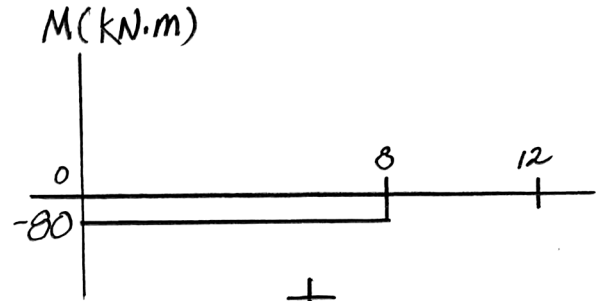
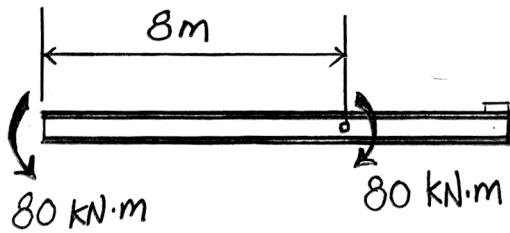
4-54. Draw the moment diagrams for the beam using the method of superposition. Consider the beam to be cantilevered from the pin support at A .



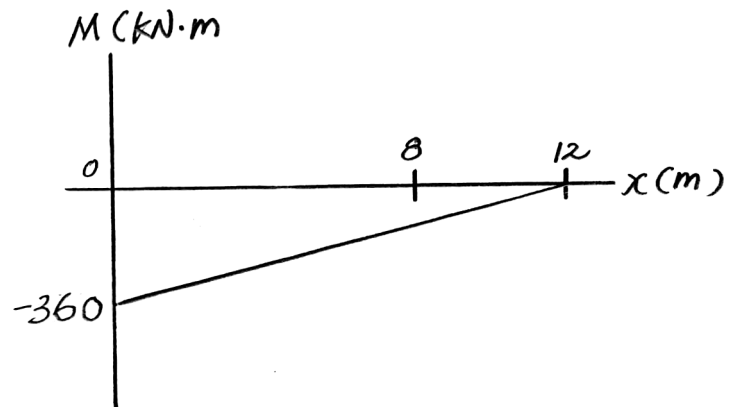
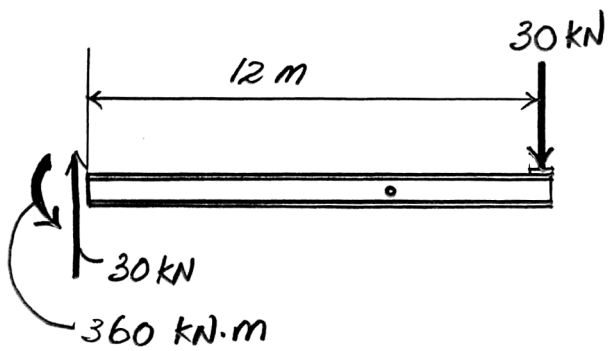
4-54. Continued



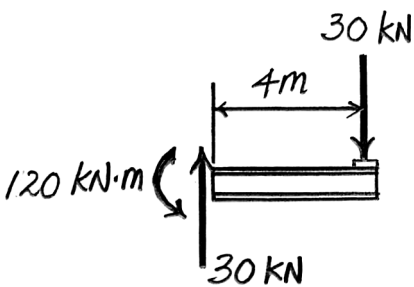
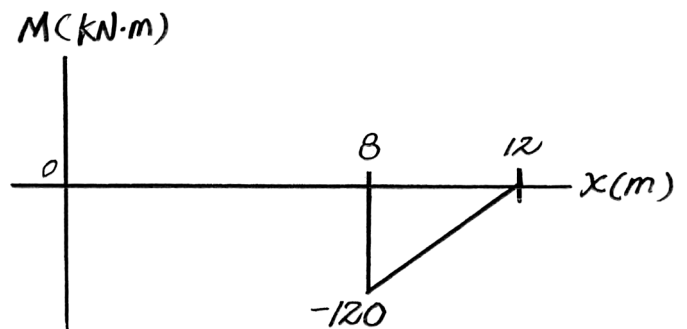
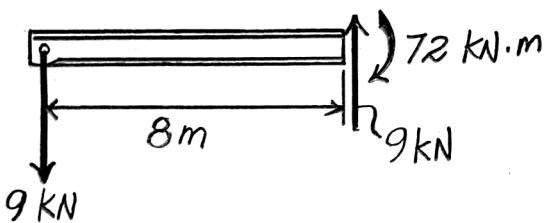
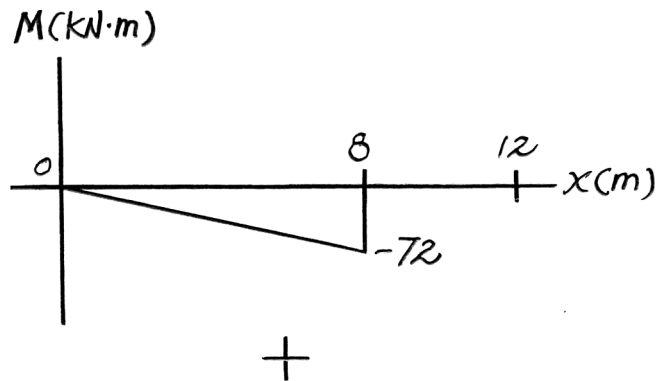
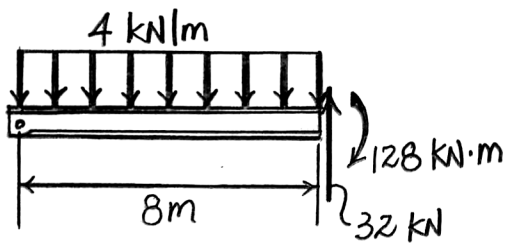
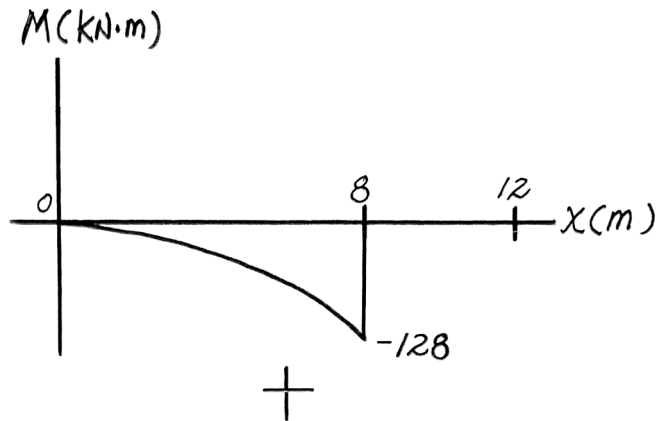
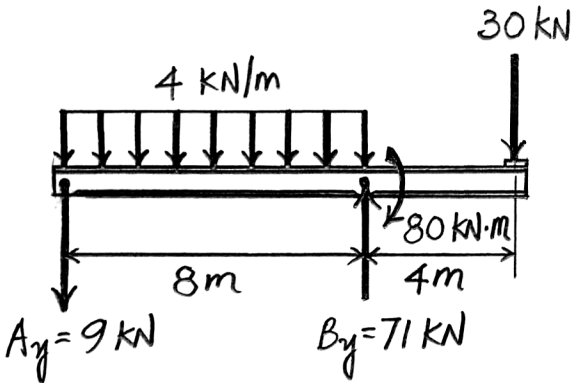
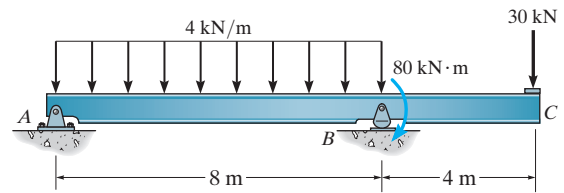
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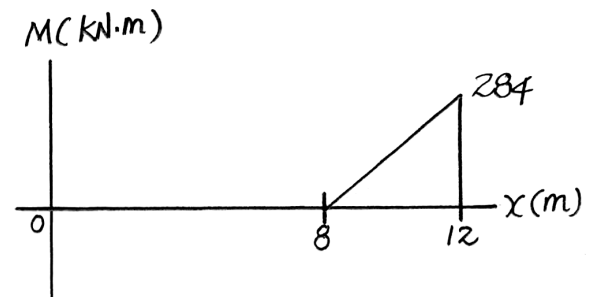
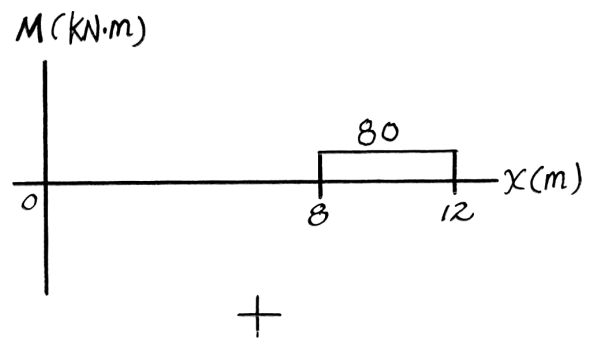
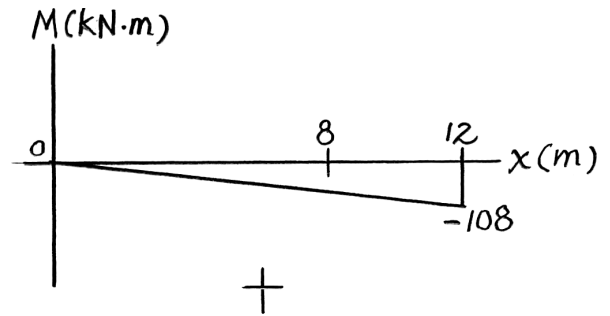
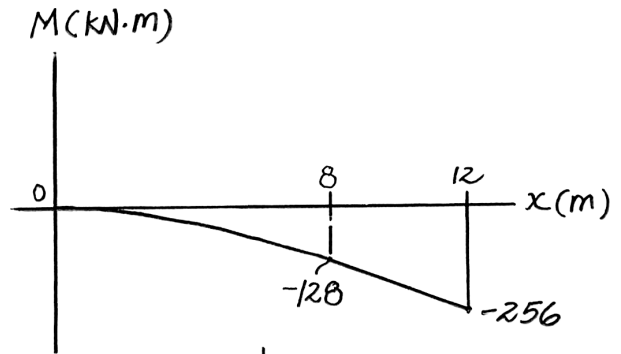
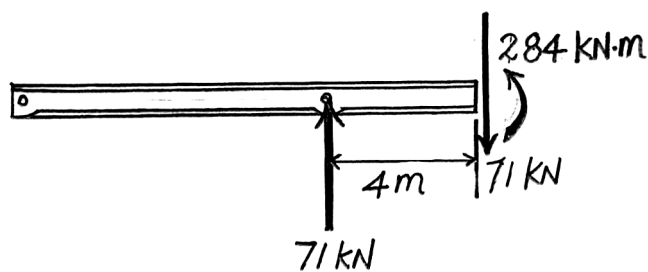
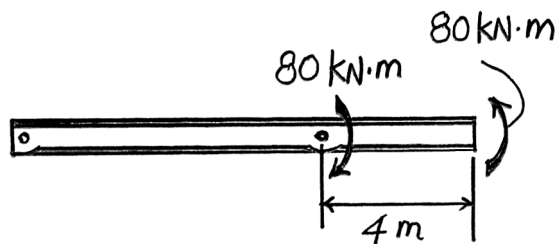
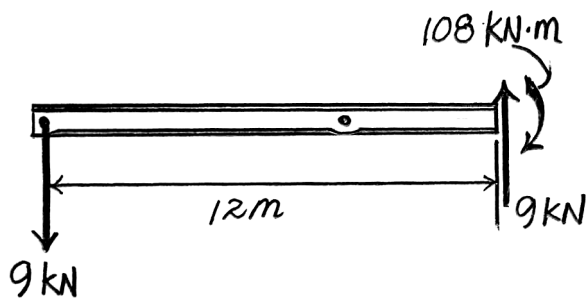
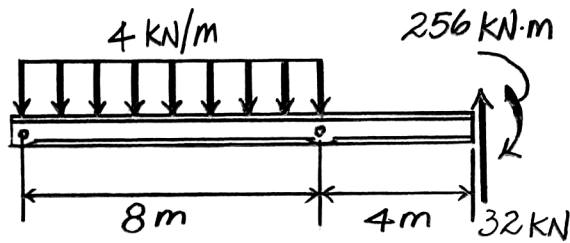
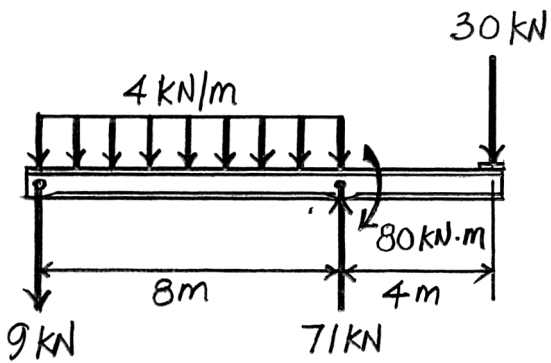
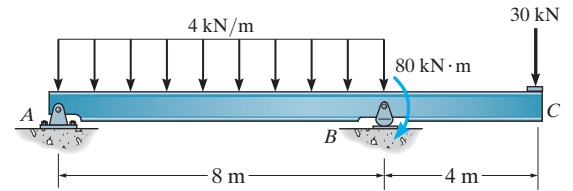
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4-55. Draw the moment diagrams for the beam using the method of superposition. Consider the beam to be cantilevered from the rocker at B.



*4-56. Draw the moment diagrams for the beam using the method of superposition. Consider the beam to be cantilevered from end C.



4-57. Draw the moment diagrams for the beam using the method of superposition. Consider the beam to be simply supported at A and B as shown.

